

Contract Administration Michael W. Chucran, Director

306 East Jackson Street, 4N Tampa, FL 33602

> Office (813) 274-8116 Fax: (813) 274-7368

ADDENDUM 2 Via E-Mail DATE: February 24, 2021

Contract 21-C-00012 Hillsborough River Dam North Embankment Remediation

Bidders on the above referenced project are hereby notified that the following addendum is made to the Contract Documents. BIDS TO BE SUBMITTED SHALL CONFORM TO THIS NOTICE.

Item 1: A site visit is scheduled for March 2, 2021 at 11:00 a.m. Interested prospective bidders should enter the Rowlett Park Main Entrance and drive to North Embankment security gate. The security gate is in the vicinity of the Dog Park. There is ample parking. One or more representatives of the Water Department will be present to provide site access.

Item 2: Section 03600-Grouting, 5, page SP-10, Delete the following paragraph:

City of Tampa

Jane Castor, Mayor

The GENERAL CONTRACTOR shall submit a description of the grouting program with the cost proposal. A description of the work procedures, ground monitoring techniques, and instrumentation program shall also be included.

Item3: Attached is a copy of the Hillsborough River Dam TO-3 Phase 2 Task 1 Geophysical Investigation Report dated February 7, 2019.

Item 4: Attached is a copy of the Hillsborough River Dam Final Engineering Report dated May 24, 2019.

All other provisions of the Contract Documents and Specifications not in conflict with this Addendum shall remain in full force and effect. Questions are to be e-mailed to Contract Administration@tampagov.net.

Jim Greiner

Jim Greiner, P.E., Contract Management Supervisor

tampagov.net -



Memorandum

Date: February 7, 2019

To: John A. Rañon, P.E., Engineer III, Water Department

From: Joseph M. Ruperto, P.E.

cc: Les Bromwell, P.E./Wood; Mark Chomtid, P.E./Wood; Ramon Martinez, P.E./URS

Subject: Hillsborough River Dam TO-3 Phase 2 Task 1 Geophysical Investigation Report

The attached report documents the geophysical field investigations conducted at the Hillsborough River Dam North Embankment as part of TO-3 Phase 2 Task 1. Included with this report is a recommendation for proposed actions to be carried out in Task 2 of this project.

URS Corporation 7650 West Courtney Campbell Causeway Tampa, FL 33607-1462 Tel: 813.286.1711 Fax:813.286.6587 www.urscorp.com



February 7, 2019

To: URS operating as AECOM

Joseph Rupert, AECOM Ramon Martinez, AECOM

From: Wood E&IS

Re: Memorandum Letter Regarding Geophysical Investigations and Proposed Action North Embankment Studies Phase II of Hillsborough River Dam, Tampa Florida Wood Project No. 300881x3

As part of ongoing dam safety efforts, URS convened a Diagnostic Assessment Workshop on 24 and 25 September 2013. The assessment team identified and discussed a number of dam safety issues related to the dam. Recommendations and conclusions documented in the workshop proceedings described a number of investigations and tasks that should be conducted to assess and improve the safety of the facility. The assessment team cited a lack of design information and geotechnical data regarding fill materials and foundation conditions at the North Embankment Dam and recommended that a geotechnical engineering evaluation be made, including investigation of a depression near the dam crest adjacent to the north retaining wall, and seepage noted exiting at ground level beyond the downstream toe of the dam. In response to a proposal submitted by URS on September 6, 2017, the City of Tampa Water Department issued Task Order 3, Phase 1, under URS General Engineering Contract to develop a better understanding of the physical conditions of the North Embankment Dam and to evaluate its stability.

The geotechnical engineering evaluation programs including 4 shallow hand auger borings, 5 SPT borings, a series of CPT soundings and seepage and slope stability analyses was performed Wood Environment and Infrastructure Solutions, Inc. (Wood) between November 2017 and June 2018. The "Final Engineering Report" presented the findings and conclusions of the Phase 1 work was issued to City of Tampa Water Department on June 14, 2018. The pertinent information for the report are presented below;

• An evidence of infilled karst feature caused by solutioning of the limestone bedrock over a long period of time was encountered in one of SPT boring located along the crest of the North Embankment.



- Three as-built cross sections were analyzed for seepage and slope stability analyses. The
 results indicated that the Factor of Safety (FS) of as-built cross sections exceed the
 required minimum FS of piping failure and slope stability for both NOWL and MSWL
 conditions.
- Wood recommend further investigation of the karst feature found in SPT boring located along the crest of the North Embankment. The investigation was to include geophysical testing and additional subsurface exploration to ascertain the extent and stability of the karst feature, and the potential need for a grouting program to ensure the future stability of this subsurface anomaly and the overlying embankment.

On August 18, 2018, the City of Tampa Water Department authorized URS to perform additional geophysical testing as part of an expanded subsurface exploration program, which was conducted under Master Consulting Services Subcontract Task Order No. 104033. The additional geophysical testing initially consisted of an Electrical Resistivity Imaging (ERI) survey performed by GeoView, Inc. (GeoView) on behalf of Wood as a part of the North Embankment Phase II study. An initial survey was carried out on August 2 and 16, 2018 within the footprint of the north portion of the Hillsborough River Dam. The purpose of the investigation was to help characterize near-surface geological conditions and to identify subsurface features that may be associated with paleo-karst activity. The pertinent information from the ERI surveys is provided below:

- On August 2, 2018, GeoView collected data from four electrical resistivity imaging transects, as shown in Figure 1. The ERI data were analyzed using AGI EarthImager 2D, a computer inversion program, which provides a two-dimensional vertical cross-sectional resistivity model (pseudo-section) of the subsurface. The results from some of the four ERI transects performed were of poor quality as evidenced by high error percentages calculated during the inversion modeling process. Based on the known geology obtained from the previous borings, the ERI data should have been of a much higher quality. Particularly, ERI Transect 1 (Figure 1), located along the suspected alignment of the infilled karst feature, was of very poor quality. Unknown conductive buried structures or utilities (cultural features) within the survey area were suspected to have caused the poor results.
- On August 16, 2018, GeoView returned to the site and performed ground penetrating radar (GPR) and electrical utility locating (EUL) surveys. Multiple electrical utilities were identified within the survey area. It was found that the most of the ERI Transect 1 had been located parallel to a buried electrical utility. Note that the ERI method measures electrical current to calculate soil resistivity, and that any conductive buried utilities (active or non-active) or structures will interfere with the measuring the current.
- Based on the ERI survey results, GeoView concluded that ERI data from Transects 2 through 4 were of acceptable quality. As depicted in Figure 1, two ERI anomalies were identified along the downstream slope (ERI Feature 1) and downstream toe (ERI Feature



2) of the North Embankment. The ERI anomalies were characterized by the apparent breach or broad down warping of the low to moderate resistivity stratum. The ERI imaging details of these anomalies are presented in Figures 2 and 3.

- However, ERI data from Transect 1, which traversed the North Embankment crest, including the area of the suspected paleo-sinkhole identified in Wood's 2017 SPT boring (SPT-4), showed spurious results for apparent resistivity values of the soils within this transect. It is GeoView's professional opinion, within a reasonable probability, that the spurious measurements were due to parallel buried electrical utility lines in the proximity of the transect. Therefore, results from Transect 1 are not considered representative of actual subsurface geological conditions.
- GeoView recommended a supplemental geophysical investigation using the Multiple Analysis of Surface Waves (MASW) methodology in order to collect data in the vicinity of the suspect paleo-sinkhole (near ERI Transect 1). The MASW survey utilizes ground impacts that generate surface waves that can be analyzed and used to calculate 2-D shear wave velocity profiles. The MASW method is less subject to interference from electrical utilities and is a proven method in identifying large sinkhole features. These profiles would provide information to supplement that produced by the ERI method, which can be used to develop a targeted boring program to further evaluate the site's subsurface conditions.

On September 29, 2018, Mr. Rañon authorized URS (operating as AECOM) and Wood to perform a supplemental MASW survey to evaluate the extent of paleo-karst sinkhole features along the North Embankment crest area. GeoView on behalf of Wood performed the MASW survey on October 2, 2018. The MASW survey was performed on two transection lines located along the North Embankment crest (Line 1) and downstream slope (Line 2). The locations of the MASW transect lines are shown in Figure 4. The results from the MASW survey are shown in Figures 5 and 6 as described below:

- The cross sections in Figures 5 and 6 show a layer of slower shear-wave velocity (V_s ~ 300 ft/sec to 900 ft/sec) material, represented in white to blue colors, from ground surface to depths approximately 20 feet deep. This velocity range is typical for soils and is suspected to correspond to the surficial sands and clayey/silty sands encountered in previous SPT borings performed at the site.
- The surficial sands and clayey/silty sands were underlain by the higher V_s materials which are depicted in green to yellow colors and may represent the clay layer over the limestone. The limestone layer is expected to be a very high V_s material (more than 2,000 ft/sec) which is represented in red color.
- Both MASW transects indicated a notable increase in the thickness of the surficial soils near the eastern and southern end of the North Embankment as shown by the pink hatched areas on Figure 4. This is indicative of an increase in depth to the top of the limestone layer. It should be noted that the top of the limestone layer in the anomaly



areas was not encountered in the MASW transects due to the limited length of the geophone array and constrained conditions of the site.

- Anomalies were encountered on Line 1 (MASW Anomaly 1) and Line 2 (MASW Anomaly 2). The MASW imaging details of these anomalies are presented in Figures 5 and 6. These anomalies are associated with an increase thickness of surficial soils and lack of the limestone layer near the eastern and southern end of the North Embankment. It also should be noted that the MASW Anomaly 1 encompasses the paleo-karst feature encountered in Wood's 2017 SPT boring (SPT-4).
- The increased thickness of the surficial soils and/or lack of limestone layer near the eastern end of the North Embankment encountered (Anomaly 1 in Figure 4) is consistent with previous SPT borings (B-4 through B-6) performed by Professional Service Industries, Inc. (PSI) in 1998. The location and soil profiles of the PSI borings are shown in Figures 7 and 8, respectively.

Using the above information, a superimposed imaging concept was used to develop the estimated thickness of the surficial soils and the top of limestone layer elevation underlying the North Embankment, as shown on Figures 9 and 10, respectively. It was found that a thickness of 50 feet or more of surficial soils is expected along the crest of the North Embankment in the vicinity of boring SPT-4, and within the middle of the downstream slope approximately 125 feet west of the concrete retaining wall.

Based on our estimated depth to the top of limestone and the anomaly features identified in the ERI and MASW surveys, four areas were identified as potentially associated with paleo-karst features. These areas of concern are shown in Figure 11, along with proposed locations for six supplemental SPT borings.

It is Wood's opinion that the investigation conducted to date revealed concerning indicators of sinkhole activity as illustrated by the concern areas presented in Figure 11. Due to the scattered distribution of the areas, a supplemental subsurface investigation with Standard Penetration Test (SPT) borings up to 100 feet deep should be performed to further delineate the subsurface conditions, including depth to competent limestone within the areas of concern. Given these conditions, a supplemental subsurface investigation with six (6) Standard Penetration Test (SPT) borings up to 100 feet deep is recommended. This recommendation will result in changing the scope of work of "Task 2 Supplemental Geotechnical Investigation" of Task Order 3, North Embankment Studies, Phase 2 as the follows:

- Increase of anticipated SPT borings from 3 SPT borings to 6 SPT borings; and
- Increase of anticipated drilling depth from 2@100 feet SPT borings and 1@50 feet boring to 6@100 feet SPT borings.

The results of the SPT borings will be used to determine: (1) the areal extent of the remediation area; (2) the recommended remediation method; and (3) to estimate the remediation cost. A



revised scope of work and fee estimate for the "Task 2 Supplemental Geotechnical Investigation" will be submitted as a separated document to the City of Tampa Water Department for review and approval.

We trust this information is helpful to you. Please do not hesitate to contact us if you have any questions or require further information.

Sincerely,

Soutil

Suppakit Chomtid, Ph.D., P.E. Senior Geotechnical Engineer

Leslie & Bromwell

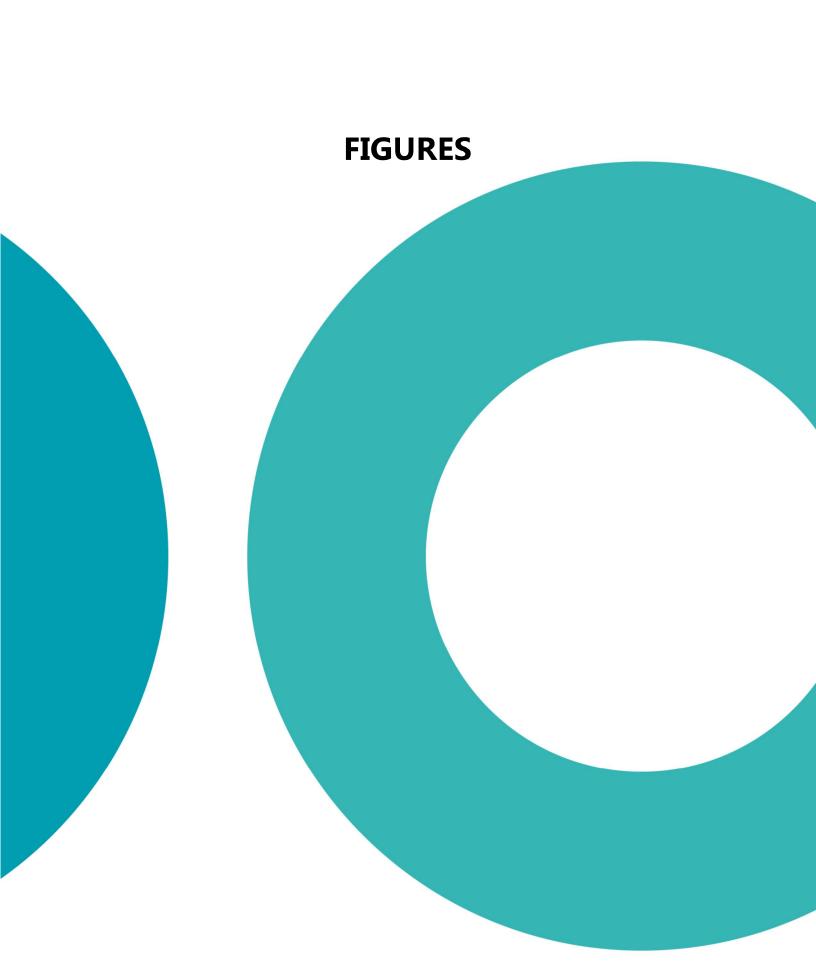
Les Bromwell, Sc.D., P.E., Principal Geotechnical Engineer

sc/lb/saw

Attachments:

- Figure 1 Location Map of ERI Transection Lines & Anomaly Features (by GeoView)
- Figure 2 ERI Imaging Details for ERI Transects 1 & 2 (by GeoView)
- Figure 3 ERI Imaging Details for ERI Transects 3 & 4 (by GeoView)
- Figure 4 Location Map of MASW Transect Lines & Anomaly Features (by GeoView)
- Figure 5 MASW Imaging Details for MASW Line 1 (by GeoView)
- Figure 6 MASW Imaging Details for MASW Line 2 (by GeoView)
- Figure 7 Location of 1998 SPT Borings by PSI (by PSI)
- Figure 8 1998 SPT Boring Profiles by PSI (by PSI)
- Figure 9 Thickness of Surficial Soil Contour Map (by Wood)
- Figure 10 Top Elevation of Limestone Contour Map (by Wood)
- Figure 11 Concerned Areas of Sinkhole Activity and Proposed Supplemental Boring Location





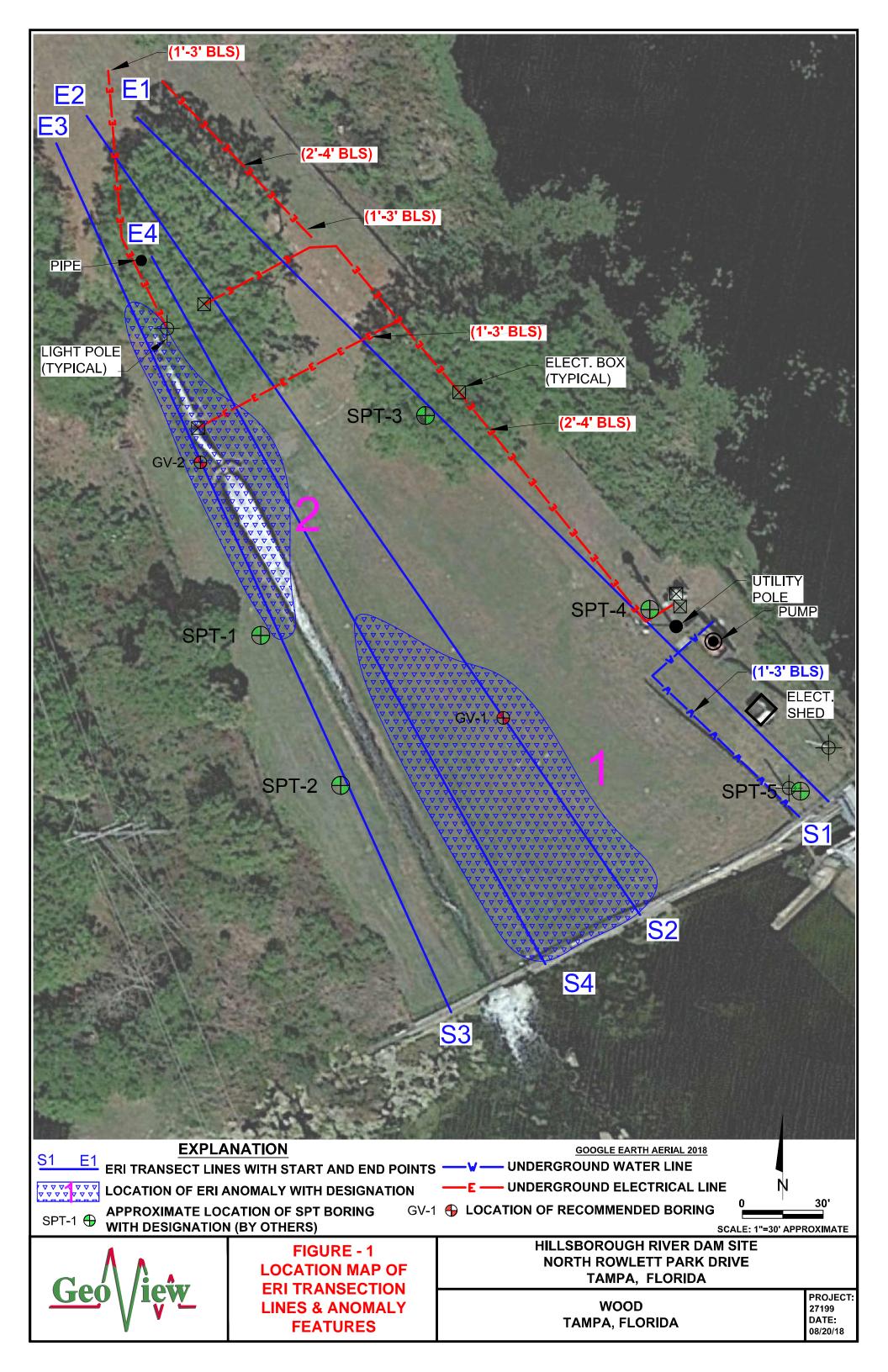
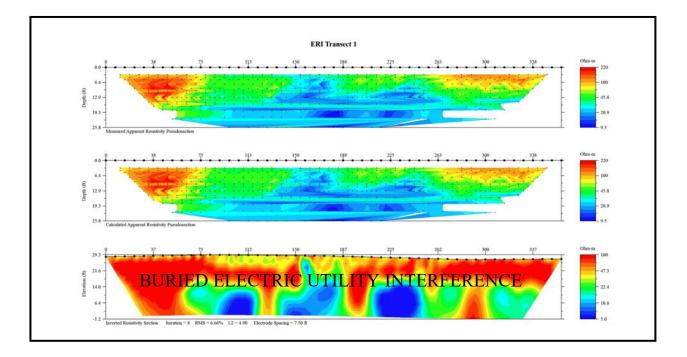


FIGURE 2: ERI IMAGING DETAILS FOR ERI TRANSECTIONS 1 & 2



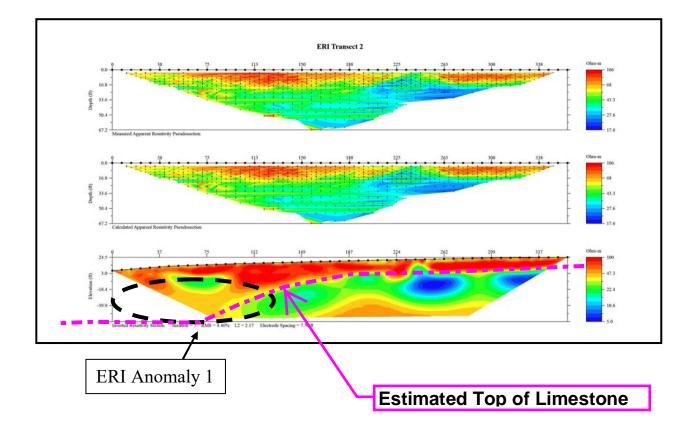
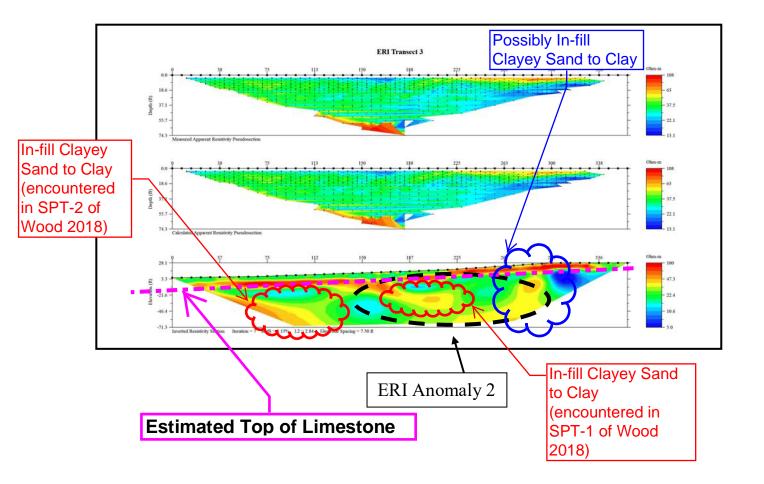
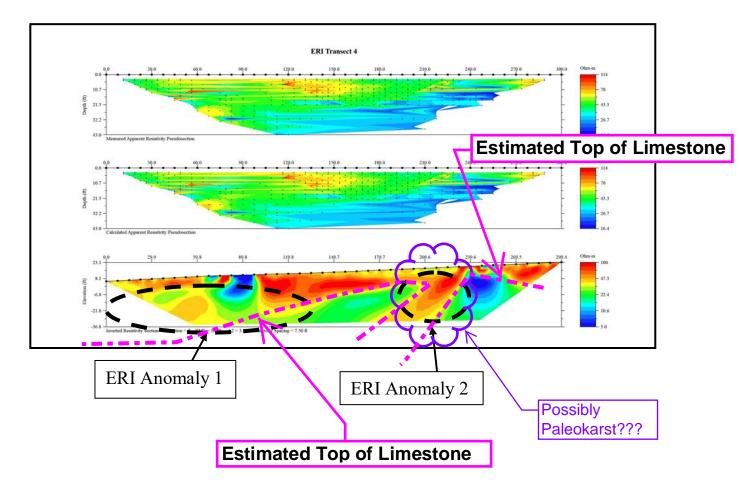


FIGURE 3: ERI IMAGING DETAILS FOR ERI TRANSECTIONS 3 & 4





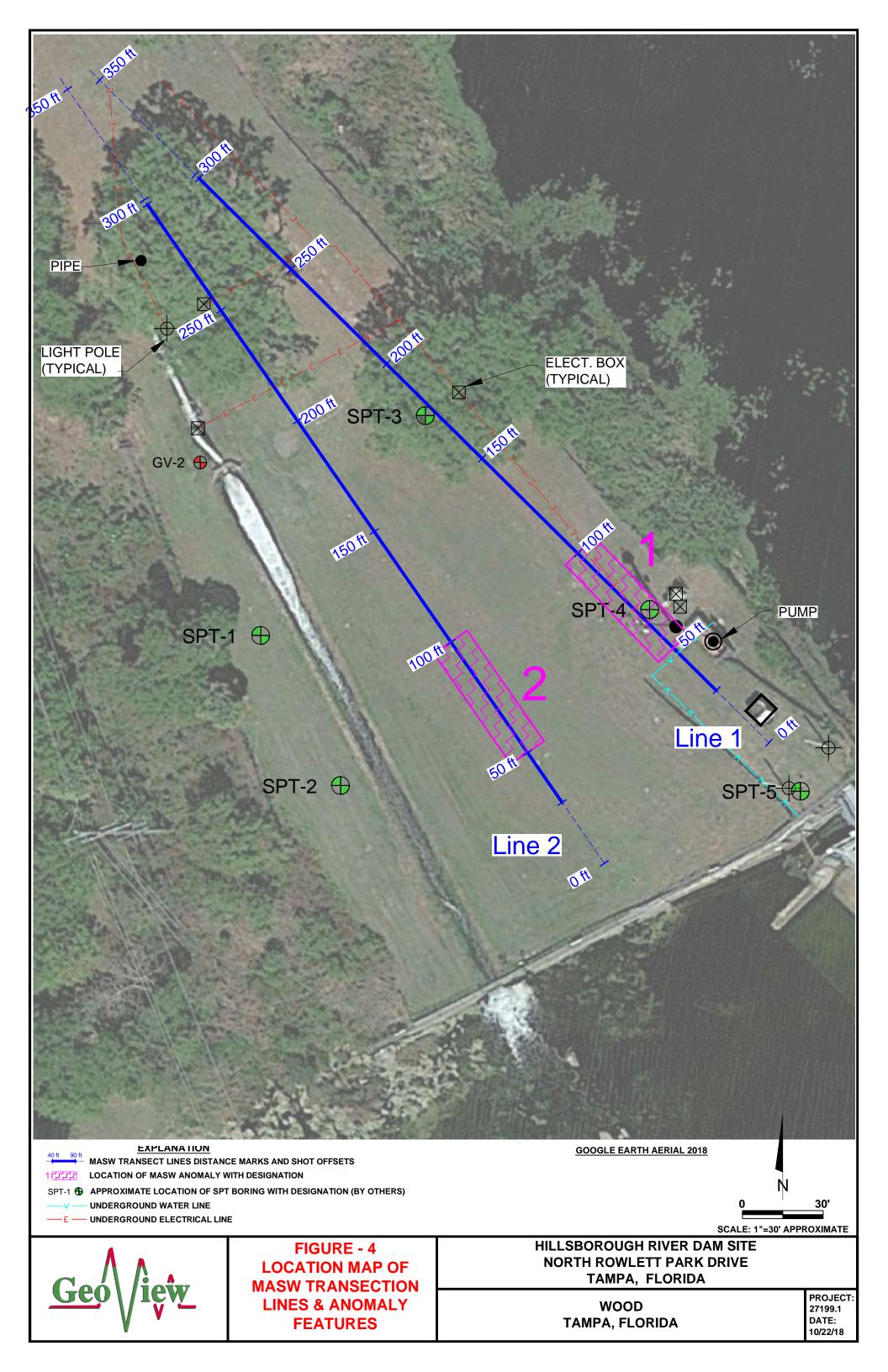


FIGURE 5: MASW IMAGING DETAILS FOR MASW LINE 1

(a) Average Dispersion Image, (b) Extracted Dispersion Curves, and (c) Shear-Velocity (Vs) Cross Section

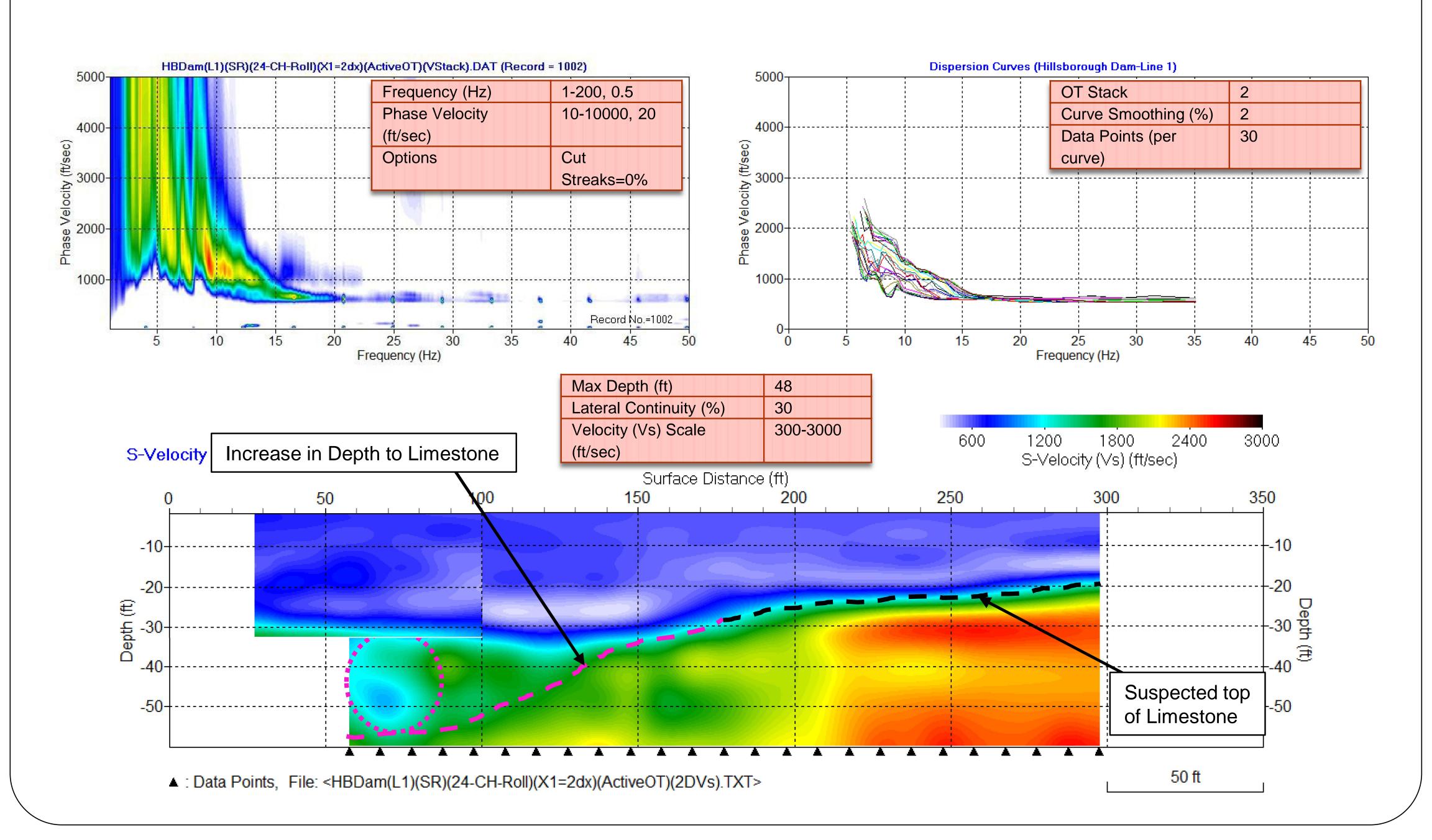


FIGURE 6: MASW IMAGING DETAILS FOR MASW LINE 2

(a) Average Dispersion Image, (b) Extracted Dispersion Curves, and (c) Shear-Velocity (Vs) Cross Section

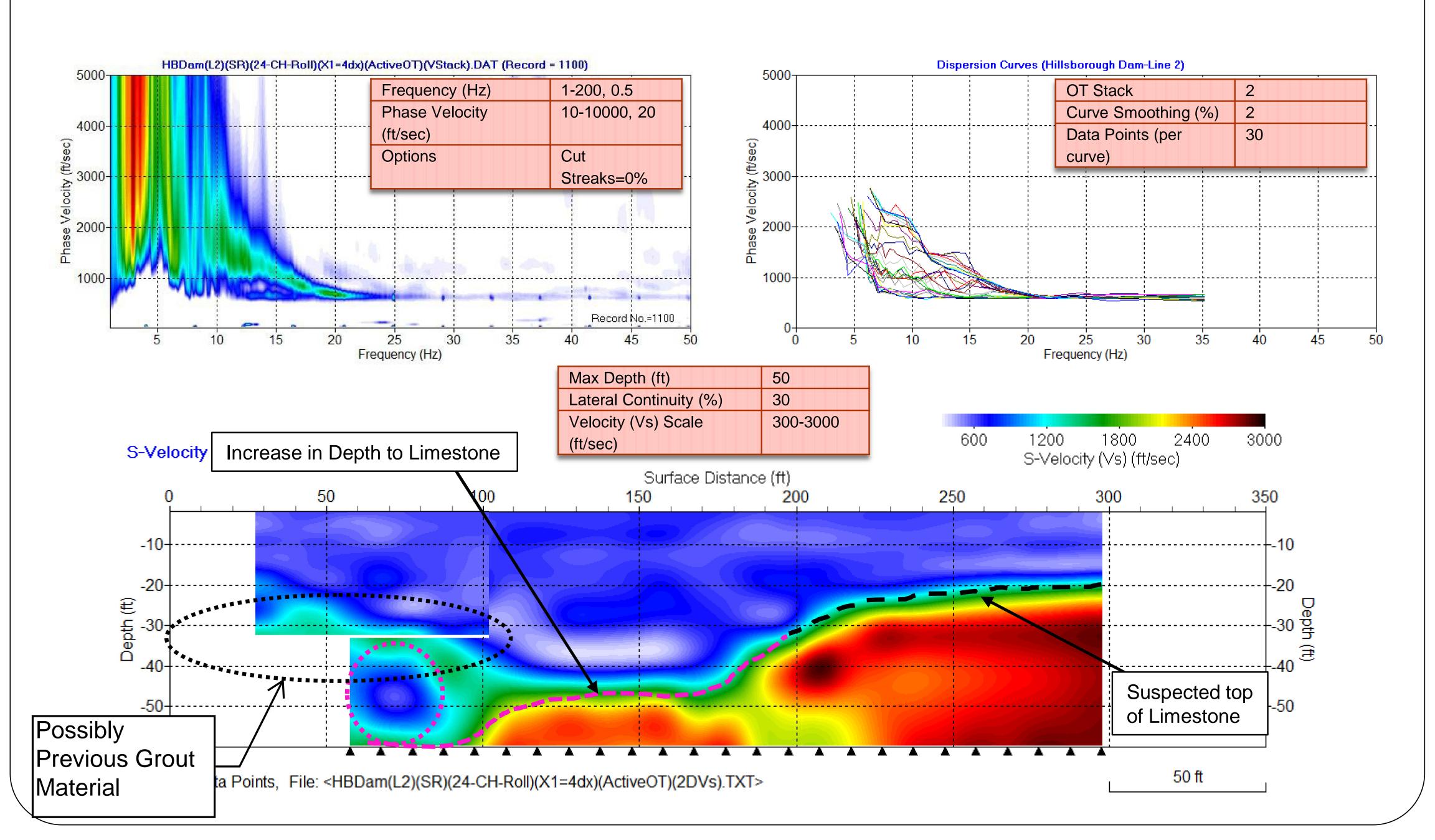
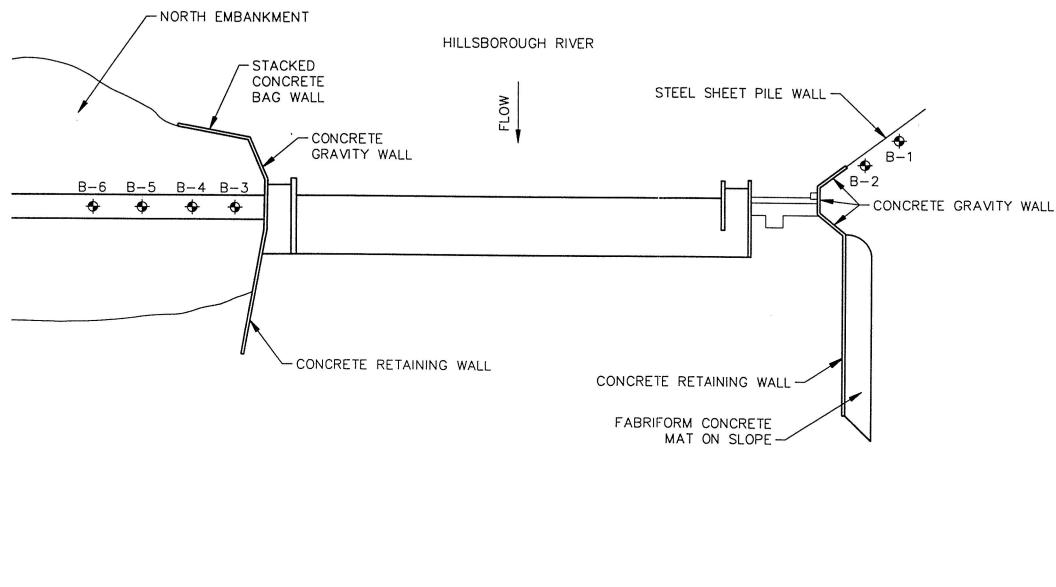


FIGURE 7: LOCATION OF 1998 SPT BORINGS BY PSI





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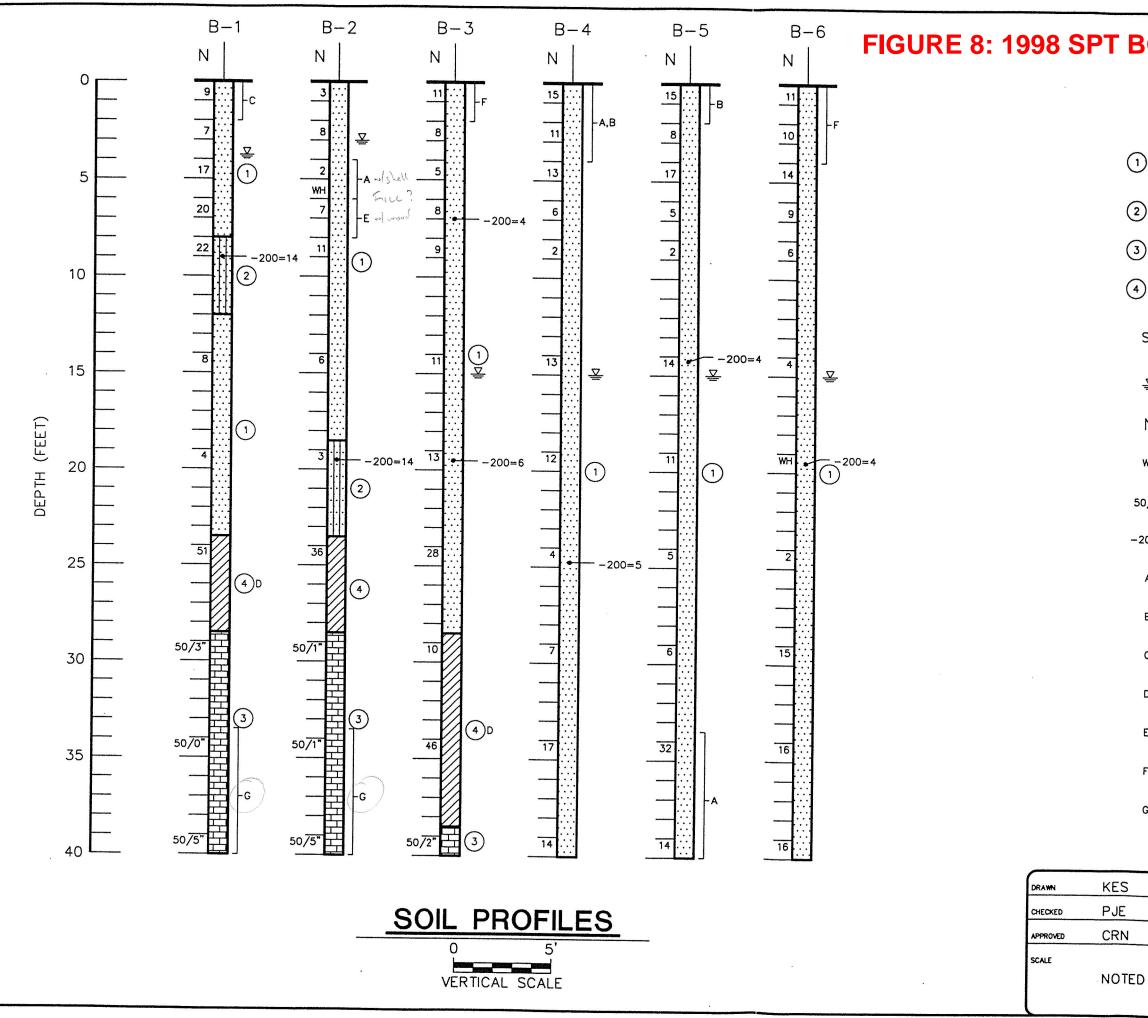


FIGURE 8: 1998 SPT BORING PROFILES BY PSI LEGEND () Brown/gray to tan slightly silty fine SAND (SP/SP-SM) 2 Dark to light brown silty fine SAND (SM) 3 Weathered LIMESTONE Tan to light green CLAY (CL/CH) Unified Soil Classification System (ASTM D 2487) SP group symbol as determined by visual review Groundwater level, October 1997 N SPT N-value in blows/foot Fell under weight of rod & hammer WH 50/6" Fifty blows for six inches Fines passing No. 200 sieve (%) -200 With shell fragments Α With clay lenses В With slight roots С D With limestone fragments With wood E With rocks F With chert G GEOTECHNICAL SERVICES HILLSBOROUGH RIVER DAM TAMPA, FLORIDA ENVIRONMENTAL GEOTECHNICAL CONSTRUCTION

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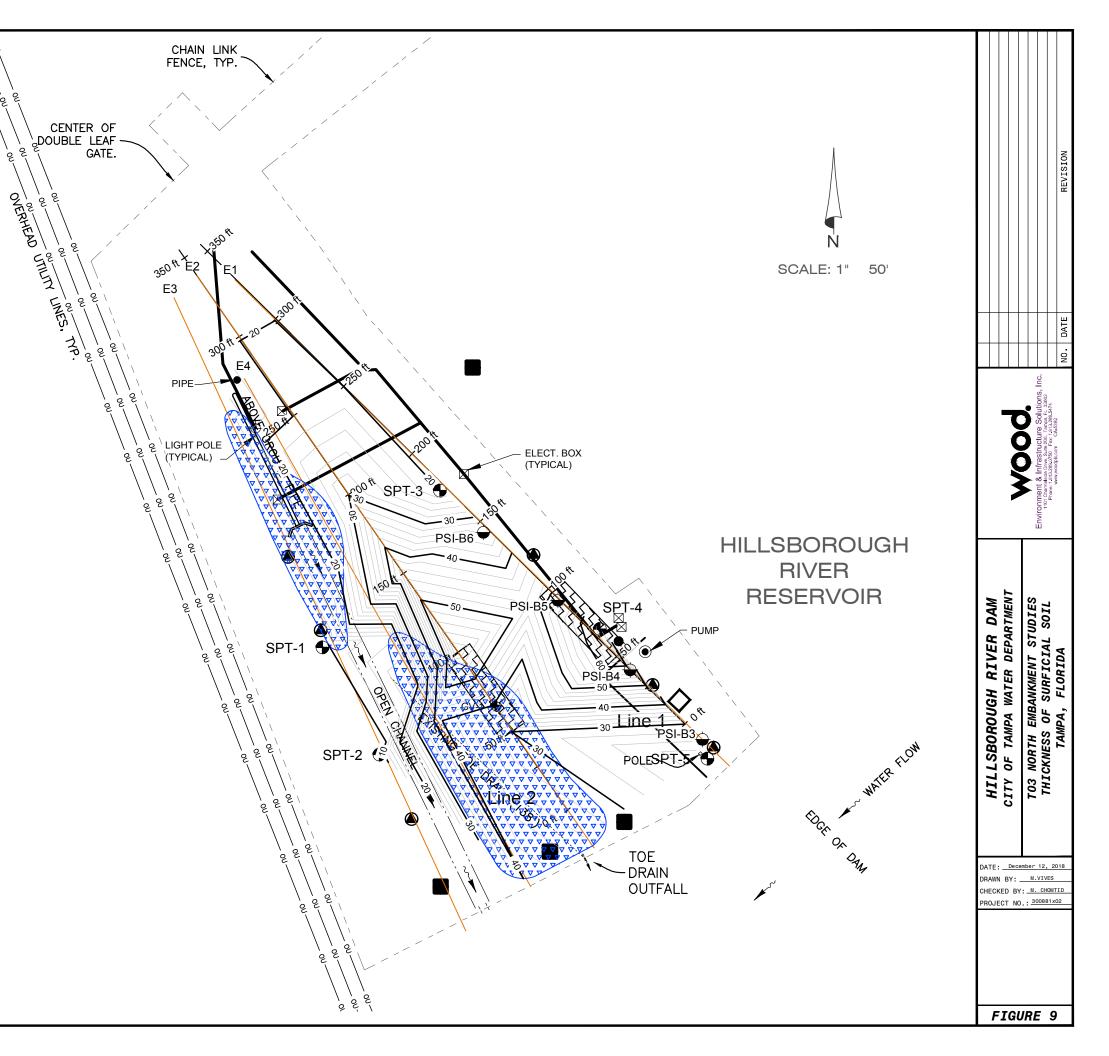




ERI ANOMALY FEATURE

- HAND AUGER BORING LOCATIONS (WOOD 2017)
- CPT SOUNDING LOCATIONS (WOOD 2017)
- ➡ SPT BORING LOCATIONS (WOOD 2017)



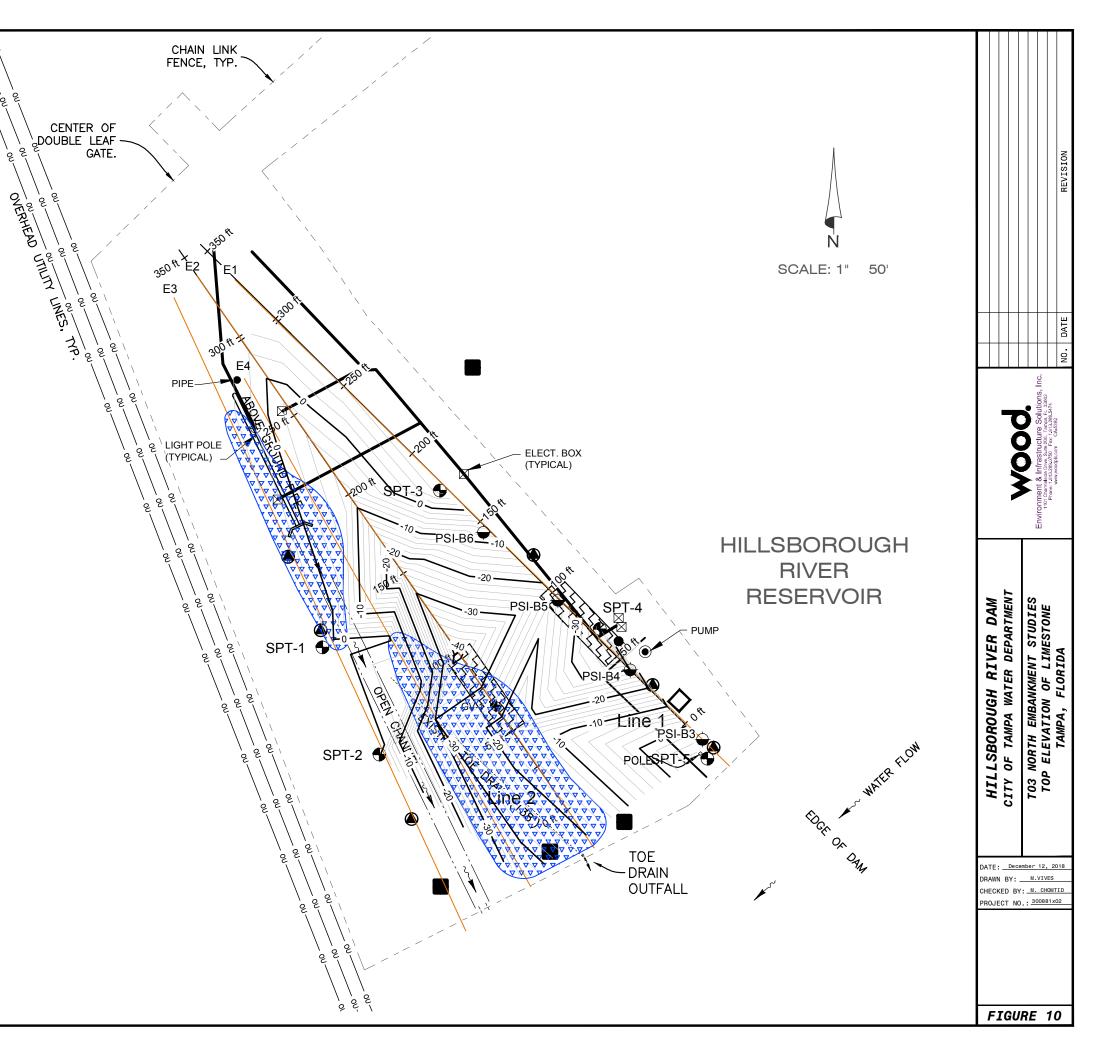




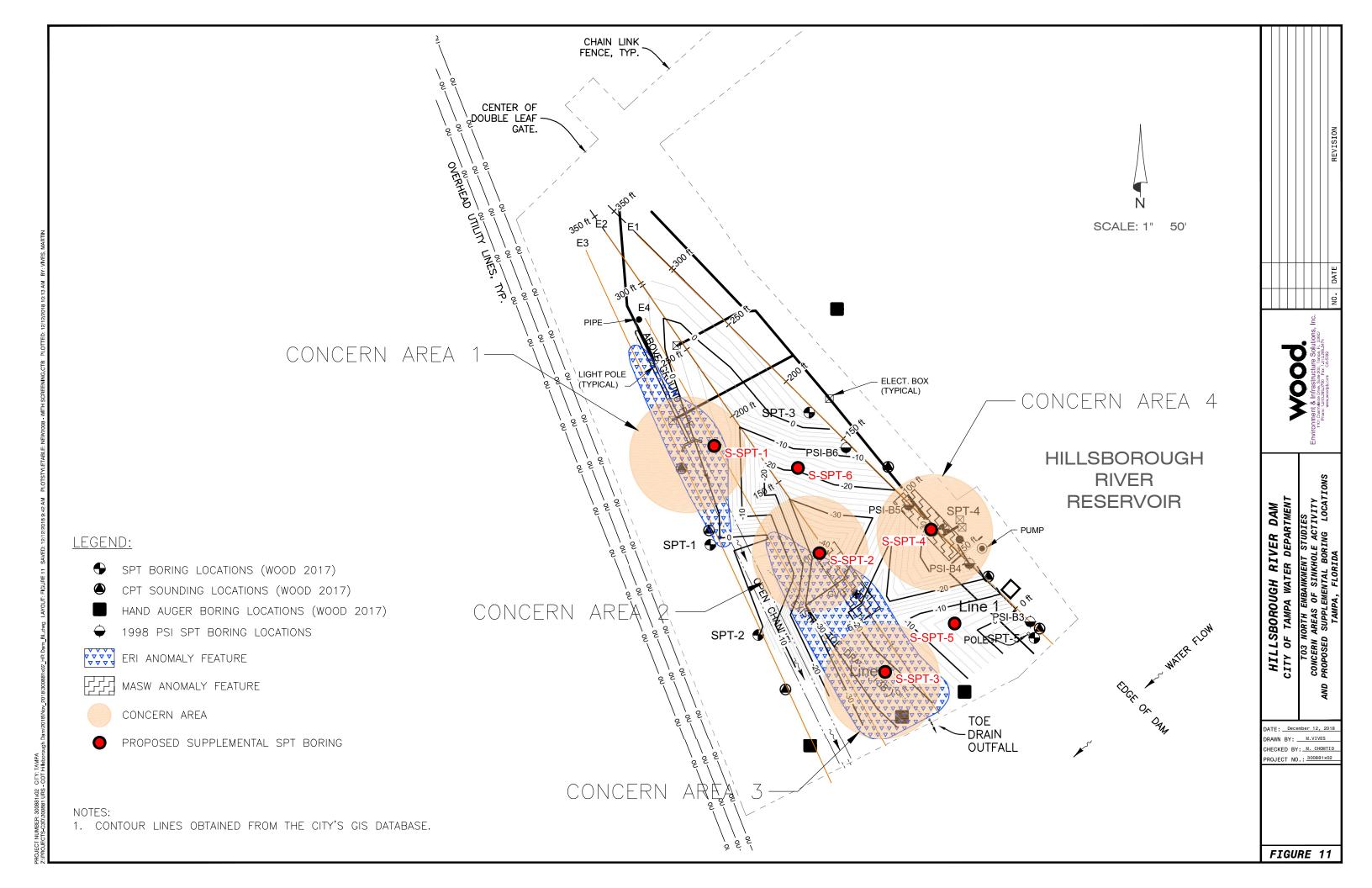
MASW ANOMALY FEATURE

ERI ANOMALY FEATURE

- HAND AUGER BORING LOCATIONS (WOOD 2017)
- CPT SOUNDING LOCATIONS (WOOD 2017)
- SPT BORING LOCATIONS (WOOD 2017)



LEGEND:





HILLSBOROUGH RIVER DAM FINAL ENGINEERING REPORT

TASK ORDER 3 - NORTH EMBANKMENT STUDIES PHASE 2

Prepared for:

CITY OF TAMPA WATER DEPARTMENT Hillsborough County, Florida

Prepared by

Wood Environment & Infrastructure Solutions, Inc. 1101 Channelside Drive, Suite 200 Tampa, Florida 33602

In Conjunction With:



7650 W. Courtney Campbell Causeway, Suite 700 Tampa, Florida 33602-1462

Wood Project No. 300881x3

May 24, 2019



CERTIFICATION Tampa

Engineering Certification

I hereby certify that I am a registered professional engineer in the State of Florida practicing with Wood Environment & Solutions, Inc., 1101 Channelside Drive, Suite 200, Tampa, FL 33602, a corporation authorized to operate as a business providing engineering consulting services (5392) by the State of Florida Department of Professional Regulation, Board of Engineers. I further certify that I, or others under my direct supervision, have prepared the geotechnical engineering evaluations, findings, opinions, calculations, conclusions or technical advice hereby represented in this report.

Luis A. Garcia, P.E.
76613
Leslie & Bromwell
Les Bromwell, Sc.D., P.E.
18234
5/24/2019

Hillsborough River Dam Final Engineering Report. Task Order 3 – NorthReport Title:Embankment Studies Phase 2

Wood Project No: 300881x3

May 24, 2019

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- APPENDIX B Soil Profiles Prepared by PSI (January 1998)
- APPENDIX C Laboratory Data



1.0 BACKGROUND AND INTRODUCTION

During the past two years, in conjunction with URS, WOOD, has conducted studies of the physical conditions of the North Embankment Dam at the Hillsborough River Dam. The embankment was constructed in 1897 of earth fill. Although very little is known regarding the details of its construction, there is no record of significant issues or incidents involving the embankment since its construction.

As part of ongoing dam safety efforts by the Tampa Water Department, URS was retained to convene a Diagnostic Assessment Workshop on 24 and 25 September 2013. The assessment team identified and discussed a number of dam safety issues related to the dam. Recommendations and conclusions documented in the workshop proceedings (Silva, 2014) described a number of investigations and tasks that should be conducted to assess and improve the safety of the facility. The assessment team cited a lack of design information and geotechnical data regarding fill materials and foundation conditions at the North Embankment Dam and recommended that a geotechnical engineering evaluation be made.

In response to a proposal submitted by URS on September 6, 2017, the City of Tampa Water Department issued Task Order 3, Phase 1, under Agreement 14-D-0047 for professional engineering services between the City of Tampa and URS Corporation Southern to develop a better understanding of the physical conditions of the North Embankment Dam at the Hillsborough River Dam.

The work consisted of Standard Penetration Test (SPT) borings, CPT soundings, shallow hand auger borings, and laboratory testing of recovered samples to determine engineering parameters. Geotechnical analyses, including seepage and slope stability modeling, were performed.

The final Phase 1 engineering report was issued to the City of Tampa Water Department in June 2018. The Phase 1 investigation and evaluation concluded that the factor of safety against slope instability exceeds the minimum requirement of 1.5, and that there was no indication of instability of the downstream retaining wall. The Study also concluded that the calculated exit gradients for seepage at the downstream toe are acceptable for both normal and maximum operating water levels. Seepage through the embankment breaks out near the toe and the existing drain system along the downstream slope is partially blocked and not capable of controlling and collecting the seepage. Although current seepage conditions are not a dam safety issue, if it becomes necessary to mitigate the seepage, an adequate toe drain should be designed and installed to prevent seepage outbreaks

During the Phase 1 field investigation an infilled karst feature, caused by solutioning of the limestone bedrock over millenniums of time, was encountered in one of the SPT borings along the crest. Such in-filled paleo-karst features can reactivate, resulting in cover subsidence sinkholes, which generally settle slowly over time and can cause significant damage to



structures. The report recommended that the karst feature be further evaluated to determine if it is stable, or if remedial measures are required in order to stabilize it.

Based on the Phase 1 results, Wood recommended a Phase 2 investigation involving geophysical testing to identify the extent of anomalous subsurface conditions in the vicinity of the karst feature, which could lead to sinkhole activity.

In response to a proposal submitted by URS on January 15 of 2019, the City of Tampa Water Department issued Task Order 3, Phase 2, under Agreement 14-D-0047 for professional engineering services between the City of Tampa and URS Corporation Southern to develop a better understanding of the physical conditions of the North Embankment Dam. The results of these additional site investigations helped to develop a better characterization of the geological conditions at the site and to better identify the extent and properties of the karst feature identified in the Phase 1 work.

This report presents the results of the Phase 2 investigation and provides conclusions and recommendations for remediation of the karst feature.

1.1 Purpose and Scope of Work

Phase 2 of the North Embankment Dam Investigation involved the following tasks:

- Task 1: <u>Perform Geophysical Investigation</u>. The geophysical testing was conducted by Geoview, Inc. and consisted of an Electrical Resistivity Imaging (ERI) survey, supplemented by a Multi-Channel Analysis of Surface Waves (MASW) investigation. The MASW technology is a seismic exploration technique that evaluates ground <u>stiffness by</u> <u>measuring shear-wave velocity</u> of the subsurface at depths up to 30 meters. The seismic source is typically a sledge hammer or similar impact device.
- Task 2: <u>Supplemental Geotechnical Investigation</u>. Following the geophysical investigations, Wood performed six (6) additional SPT borings to help define the depth and extent of the karst feature, depths of the borings varied between 50 and 150 feet below ground surface. This task also included laboratory index testing on selected soil samples for classification and evaluation of remediation options.
- Task 3: <u>Final Engineering Report and Remediation Design</u>. The results of the ERI and MASW investigations are summarized herein. Conceptual recommendation alternatives and approximate cost estimates for the remediation have been developed and are presented.



2.0 PREVIOUS EXPLORATIONS AT THE SITE.

The previous site investigations performed at the site are as follows:

1. Geotechnical Engineering Services Embankment and Foundation Evaluation Hillsborough River Dam. Prepared on January 1998 by Professional Service Industries (PSI).

This investigation included six (6) SPT borings in the embankment and abutment areas and engineering analyses to verify the stability of the North Embankment and to evaluate the south wing wall. The PSI borings have been incorporated into this report.

2. Hillsborough River Dam Final Engineering Report – Task Order 3. North Embankment studies Phase 1. Prepared in June 2018 by Wood.

This investigation included, five SPT borings, 6 CPT soundings, and four shallow hand auger borings, along with laboratory testing and seepage and slope stability analyses. Wood recommended additional investigation of a karst feature identified along the crest of the North Embankment. The recommended additional investigation included geophysical tests and additional subsurface exploration.

3. Geophysical Investigation at Hillsborough River Dam Site. Final ERI Report. Prepared by GeoView, on behalf of Wood, on September 5th, 2018.

This additional investigation consisted of an Electrical Resistivity Imaging (ERI) survey within the footprint of the north portion of the Hillsborough River Dam. The goal was to characterize near-surface geological conditions and to identify subsurface features that may be associated with karst activity. GeoView recommended a supplemental geophysical investigation using the Multi-Channel Analysis of Surface Waves (MASW) methodology in order to collect additional data around the anomalous area.

4. Geophysical Investigation at Hillsborough River Dam Site. Final MASW Report. Prepared by GeoView, on behalf of Wood, on October 22nd, 2018.

The Multi-Channel Analysis of Surface Waves (MASW) seismic investigation consisted of two transect lines, one located along the embankment crest and the other on the downstream slope. Using the MASW results and the findings from the previous explorations, four (4) areas were identified as associated with the karst feature. These are indicators of in-filled paleo-karst activity. Due to the scattered distribution of the areas, a supplemental subsurface investigation consisting of six supplemental SPT borings was proposed in order to better define the extents of the feature.



3.0 SUPPLEMENTAL SUBSURFACE INVESTIGATION AT THE SITE

Standard Penetration Tests (SPT)

The six supplemental SPT borings were completed between February 11 and 21 of 2019. **Figure 1** shows the location of these SPT borings. **Figure 2** shows all of the SPT borings at the North Embankment dam, including those from Wood Phase 1 (2018) and PSI (1998).

The soils encountered in the borings were visually classified and logged in the field in accordance with ASTM D 2488 (Visual-Manual Procedures). Representative portions of the samples were transported to Wood's materials testing laboratory in Tampa, Florida for further classification and testing. The logs were then updated based on the results of the laboratory classification tests (see **Section 5.0**) using the Unified Soil Classification System (USCS) in accordance with ASTM D 2487.

The subsurface conditions encountered are presented on the supplemental soil boring logs (S-SPT-1 to S-SPT-6) in **Appendix A**. **Appendix A** also includes the previous borings performed by Wood in 2018 (SPT-1 to SPT-5). On completion, the SPT boring locations were surveyed to determine their coordinates using a handheld Garmin GPS model 72H with a reported accuracy of +/- 10 feet. The locations should therefore be considered as approximate. The ground surface elevations were estimated using the GIS contour lines obtained from the City of Tampa database.

The boring logs represent our interpretation of the subsurface conditions encountered in the field, the visual examination of field samples by our technical staff, and the results of our laboratory testing program. The lines designating the interfaces between various strata on the boring logs represent approximate interface locations. Actual transitions between strata may be gradual.

The SPT borings were performed by Madrid Engineering Group, Inc. (MEG). The drilling was completed using a track mounted drill rig. The SPT borings were conducted in general accordance with ASTM D 1586 using mud-rotary drilling methods. The top 4 feet of each SPT boring were drilled using hand augers. SPT testing was conducted continuously between 6 feet and 10 feet below ground surface and on 5-foot centers thereafter. Soil samples were collected from the borings using a 1.5-inch inner diameter split-spoon sampler driven with an automatic 140-pound slide hammer falling a distance of 30 inches. The soils from the recovered split spoon samples were visually logged in the field and the sample color was identified using Munsell color charts.

The six SPT borings were advanced to depths between 50 and 150 feet below ground surface. At the end of drilling, each SPT boring was filled with cement-bentonite grout.



Table 1 includes a list of the supplemental SPT borings performed at the site.

Boring ID	Date Completed	Northing (ft)	Easting (ft)	Ground Elevation (NAVD 88, ft)	Boring Termination Depth (ft, BGS)
S-SPT-1	02/12/2019	1342133.8	517614.3	+18.0	50
S-SPT-2	02/14/2019	1342053.3	517695.5	+16.0	80
S-SPT-3	02/14/2019	1342006.0	517714.4	+10.0	70
S-SPT-4	02/20/2019	1342094.8	517738.2	+28.0	110
S-SPT-5	02/21/2019	1342049.5	517783.6	+21.0	55
S-SPT-6	02/19/2019	1342123.7	517664.8	+21.0	150

TABLE 1. Summary of supplemental SPT borings

Notes:

Northing and Easting coordinates and elevations were measured using a Garmin GPS 72H model and rounded to the nearest tenth. Northing and Easting coordinates are referenced to the US State Plane, Florida East Zone (901).

BGS = below ground surface.

Ground Elevations were estimated using the GIS contour lines obtained from the City of Tampa database.

Appendix B presents the January 1998 boring logs prepared by Professional Services Industries (PSI). Note that PSI borings B-1 and B-2 were not drilled at the North Embankment Dam.

4.0 SUBSURFACE CONDITIONS

Based on the findings of the subsurface investigations and laboratory testing, four generalized soil strata were identified at the site. **Figures 3** and **4** show the locations of the cross sections that were used to prepare the soil profiles presented in **Figures 5**, **6** and **7**. The strata are distinguished by physical characteristics, typically grain size and plasticity. These units are described below.

Stratum 1	EMBANKMENT FILLS AND UNDIFFERENTIAL SURFICIAL SOILS.
Stratum 2	CLAYEY SANDS (SC) AND SANDS (SP), (SP-SM).
Stratum 3	LIMESTONE with trace calcareous CLAYS (CL).
Stratum 4	INFILL CLAYS: Calcareous SANDY CLAYS (CL) and CLAYS (CL) with fragments of limestone.

Stratum 1 is composed of embankment fills and undifferentiated surficial soils, sampled as SANDS (SP) to SANDS with CLAYS (SP-SC) to CLAYEY SANDS (SC) to SANDS with SILTS (SP-SM) to SILTY SANDS (SM). The color of the soils in this stratum varied from light brown to brown to light gray to gray to dark gray to orange brown to light orange brown. Also, this stratum



presented trace limestone fragments, trace shell fragments and trace organics. The depth of this stratum varied between 12 to 32 feet below the ground surface depending on the location on the slope of the embankment. The blow counts varied between 3 and 63 blows per foot, presenting very loose to very dense consistencies. In the in-filled karst zones the blow counts indicated very loose sands to very soft clays.

Stratum 2 consists of light gray to gray to dark gray to greenish gray to tan to light brown CLAYEY SANDS (SC) and SANDS (SP) with blow counts (N) varying from 3 to refusal, resulting in consistencies from very loose to very dense. This stratum varies in thickness from 5 to 20 feet and was encountered between 12 and 82 feet beneath the surface. Laboratory test results shown in **Appendix C** indicate moisture contents between 19% and 39%, fines (passing No. 200 sieve) content between 20% and 45% and plasticity index between Non-plastic (NP) and 19. These soils have infilled and replaced solutioned limestone in the karst feature. Some of the borings exhibited high blow counts, indicating stable in-filled conditions. Others, particularly in the vicinity of SPT borings SPT-4, S-SPT-4, S-SPT-6, and PSI borings PSI-B4, B-5, and B-6 exhibited low blow counts typical of raveled soils in active karst features.

Stratum 3 is composed of LIMESTONE with trace calcareous CLAY (CL). The unit consists of light gray to brown fragmented limestone with in-filled calcareous clay (CL). The blow counts present SPT-N values varying between 0 blows per foot to refusal, indicative of very weathered to hard limestone and very soft to hard clay. The thickness of this stratum varied between 5 and 13 feet and was encountered between 12 feet and 102 feet below surface. This limestone stratum was not encountered in supplemental borings S-SPT-2, S-SPT-3 nor S-SPT-4. It also was not encountered in previous boring SPT-4, or in PSI borings B-4, B-5, and B-6. The absence of limestone indicates the progression of weathering and solutioning over long periods of time. Estimated rates of solutioning of limestone in central Florida are in the range of 0.5 to 1 inch in 1,000 years (Lane, 1986).

Stratum 4 is composed of infill clays sampled as calcareous SANDY CLAYS (CL) and CLAYS (CL) with fragments of limestone. This unit consists of light gray to gray to dark gray to tan to light brown to blue gray sandy clays (CL) and clays (CL) with fragments of limestone. The thickness of this stratum varied between 5 and 43 feet and was encountered between 17 and 107 feet below ground surface. The blow counts varied from 4 to refusal, indicating consistencies of soft to hard. Areas of the karst feature with higher blow counts, including hard or refusal, are considered to be stable. Areas that have low blow counts and typically soft clays are likely active or subject to reactivation.

General Strata Observations

• **Figures 2, 3, and 4** show the anomaly areas indicated by the ERI and MASW geophysical investigations. The figures also show the location of loose or soft in-filled soils found in the SPT borings where the limestone is either highly weathered or absent due to solutioning. This part of the in-filled karst area is highlighted as "Area of Concern." This



area of concern is also shown on the soil profiles in **Figures 5**, **6** and **7**.

- Figures 3 and 4 also show the locations of six cross sections selected to show subsurface soil and rock conditions at the North Embankment Dam. Figures 5, 6 and 7 show the cross sections, which include all of the SPT borings. It can be noticed that the top of the limestone rock, where limestone has not been completely solutioned, varies significantly between elevations +8 to -80.0 feet. This illustrates the tremendous variation in the solutioning of the limestone from one section of the dam to another. It is also worth noting that previous borings beneath the concrete dam section have indicated that the top of rock is at approximately elevation 0, with no indication of significant karst formation. However, it should be recognized that there are few borings beneath the concrete section, and they are limited to the northern part of the dam.
- Based on available information, it appears that the area susceptible to potential karst activity reactivation (Area of Concern see **Figure 2**) extends along the crest of the southern part of the Embankment for a distance of approximately 300 feet and extends down from the crest elevation of 28 feet to an elevation of 17 feet on the downstream slope.

5.0 LABORATORY TESTING

The soil samples collected during the subsurface exploration program were transported to Wood's materials laboratory in Tampa, Florida. Selected samples were tested for natural moisture content (ASTM D 2216), percent of material finer than the #200 sieve (ASTM D1140), and Atterberg Limits (ASTM D 4318). Soils were classified in general accordance with ASTM D 2487. A summary of the laboratory test results is presented in **Table 2.** Laboratory test results are also presented in **Appendix C** and are generally consistent with our field descriptions. A detailed discussion of the strata encountered in this subsurface exploration was presented in **Section 4**.

The laboratory test results were used to modify the field logs for presentation in the Boring Logs in **Appendix A**.



Stratum No.	Description	Moisture Content (%)	% Finer #200 Sieve	Liquid Limit (LL)	Plastic Limit (PL)	Plasticity Index (PI)
2	CLAYEY SANDS (SC) AND SANDS (SP)	19%-39%	20%-45%	34-37	15-24	NP-19
4	INFILL CLAYS: Calcareous SANDY CLAYS (CL) and CLAYS (CL)	51%-73%	58%-82%	54-66	18-23	31-48

TABLE 2.	. Laboratory Test Data Summary by Strata ⁽¹⁾
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⁽¹⁾ Strata 1 and 3 were not analyzed for laboratory tests.

6.0 **GROUNDWATER CONDITIONS**

The initial portion of each of the borings was advanced by dry-auger methods to depths of 4 ft below ground surface. Groundwater was not observed during these initial dry-auger borings. Following the hand augers, rotary mud drilling was utilized to the boring termination depths. Due to the wet method of drilling and since the boreholes were grouted after investigations were completed, the location of the phreatic line was not determined.

7.0 CONCLUSIONS AND RECOMMENDATIONS

Karst feature stabilization

After reviewing the investigations performed at the site, it is Wood's opinion that a portion of the infilled karst feature found at the North Embankment Dam is potentially susceptible to reactivation that could result in significant subsidence and potential breaching of the dam.

Fluctuations in water levels in the dam, due to changes in reservoir and/or downstream water levels, as well as rainfall infiltration, result in changes in the state of stress within the karst feature. Such changes can cause reactivation of infilled karst features, leading to subsidence and sinkhole formation (Smith and Horowitz, 2004).

Although a portion of the infilled karst feature exhibits low blow counts and raveling of soils, there has been no surface indication of significant subsidence. Given the age of the Dam, the feature has been stable for a long period of time. Nevertheless, the consequences of a significant subsidence event, and the inability to forecast future events, require conservative thinking and planning regarding remediation and stabilization of the feature. Our recommendation is that the City Water Department proceed with the necessary work as soon as practical.

Based on our 30+ years of experience in sinkhole evaluations and remediation, it is our opinion that the only economic alternative to stabilize the karst feature on the North Embankment is by



grouting with a cementitious Low Mobility Grout (LMG) material. Therefore, this method is recommended to stabilize the North Embankment. The intent of the grouting is to densify and stabilize the raveled soil zones, infill the weathered limestone, and seal any karst-related fractures and/or cavities with a stable material.

The recommended grouting program should consist of a series of primary vertical grout injection points in the area of concern identified by the geotechnical explorations, as shown in **Figure 8**. A LMG grout should be utilized to ensure effective treatment of the loose/raveled zones and to reduce migration of grout beyond the affected areas.

We estimate the grout quantity will be between 1,130 and 2,260 cubic yards. The LMG grout should have a slump of 3 to 5 inches at the hopper with a minimum compressive strength of 400 psi at 28 days. The recommended maximum injection pressure should be 150 psi above line pressure.

The proposed remediation plan consists of one hundred twenty-six (126) primary vertical grout injection points, on a 10 ft by 10 ft pattern, at the locations shown on **Figure 8**. We note that additional, intermediate grout points (secondary and tertiary) may be necessary if high grout volumes are recorded during the grouting program. In that case the grouting program may move outside the proposed primary grouting area until stable conditions are encountered.

The grout casings should be installed into firm limestone bedrock or a competent soil stratum, which is estimated to occur at depths between 40 and 110 feet below grade based on the SPT borings. Due to the local zones of very soft and raveled soils, appropriate drilling equipment should be utilized to ensure the grout injection pipes are installed to the required depths. During grout point installation, care should also be taken to identify the soil materials to ensure the grout pipes are not installed to depths significantly below the competent limestone surface and should be limited to a maximum of 2 feet into competent limestone. Each grouting operation should start at the bottom depth and continue upward in 5-foot increments until reaching 10 feet from the ground surface. As grouting proceeds, constant monitoring of the ground surface will be required to avoid uplifting. Grouting pressures will be reduced as necessary to prevent uplift and no grouting will be done within 10 feet of the ground surface.

During mobilization and prior to grouting, particular care should be taken to identify subsurface obstructions at the site that could potentially be damaged (such as underground pipes, utilities, etc.). In case grout point locations need to be moved, it is recommended to confirm with the owner's inspector.

During the grouting, it is recommended that the owner's inspector continue to monitor any seepage observed in the downstream portion of the North Embankment and to visually note any indications of surface movement or subsidence.

Depending upon the grout take volumes at injection points on the crest, it may be necessary to



grout on the upstream side of the dam. If upstream grouting is required, it may be necessary to either lower the reservoir water level, or mobilize a barge in order to access these grout locations.

<u>Toe Drain</u>

Figure 1 shows the location of an existing PVC toe drain on the downstream slope of the embankment, which originally was 200 feet long, and is located on the downstream side of the North Embankment. This toe drain was installed in early 1980s. An inspection of the drain on January 5, 2018 by Kissinger Campo, Inc. field technicians revealed that the pipe is plugged at a distance of 138 feet from the outlet, which is approximately where a discharge flume for water pumped from Sulphur Springs crosses the drain location.

Wood recommends performing a pipe inspection by a qualified company that can deploy a video camera inside the pipe and provide a video for engineering evaluation. The inspection should include a cleaning of the pipe, a determination of the cause of the obstruction and removal of it, if feasible.

It is worth noting that the location of the existing PVC toe drain does not interfere with the grouting locations shown in **Figure 8**.

Monitoring Wells

Monitoring wells and piezometers should be installed at the North Embankment Dam to provide ongoing information regarding changes in water levels and water pressures beneath the crest and at downstream toe of the dam. Such instrumentation will provide information regarding the future performance of the dam, and can indicate changes that may reflect the development of adverse conditions. A monitoring plan, identifying the locations and depths of the instruments, along with a cost estimate and schedule for reading the instruments, should be developed after the grouting plan is completed, in order to avoid potential damage to the instruments during grouting.

8.0 **GROUTING COST ESTIMATE**

Table 3 presents the conceptual cost estimate for the compaction grouting and **Table 4** presents the cost estimate for the grout monitoring work and as-built report:



ltem	Estimated Quantity	Unit Cost	Sub-Total
Mob/Demob for LMG Grouting	2	\$2,500	\$5,000
Installation of LMG Grout Casing	11,310 feet	\$18/LF	\$203,580
LMG Grout Injection	1130 to	\$170/CY	\$192,100 to
EMG Grout Injection	2,260 CY	\$170/Cf	\$384,200
		TOTAL	\$400,680 to
		IUIAL	\$592,780

Table 3. Conceptual Cost Estimate of the Low Mobility Grout Remediation Program ⁽¹⁾

(1) This cost estimate only considers primary grout points. A supplemental cost estimate will be prepared if secondary and tertiary grout points are required.

(2) LMG = Low Mobility Grout

Table 4. Preliminary Cost Estimate of Monitoring the Grout Remediation Program⁽¹⁾

Task / Deliverable	Unit Cost	Hours / Day	Days	Sub-Total	Comment
Principal Engineer	\$205/hr	4	4	\$3,280	Periodic Site visits
Sr. Geotechnical Engineer	\$140/hr	4	12	\$6,720	Periodic Site visits
2 Field Technicians	\$75/hr	10	115	\$172,500	Includes Grout Monitoring, Oversight and Progress Reporting
As-Built Report	\$5,000	-	-	\$5,000	Engineering Report Package (Lump Sum)
			TOTAL	\$187,500	

(1) This cost estimate only considers primary grout points. Another cost estimate will be prepared if secondary and tertiary grout points are required.

The total conceptual cost estimate for the remediation work ranges from **\$588,180** to **\$780,280.**

Note that the estimated fees of the monitoring program will vary depending on actual construction activities and schedules, which will be determined by the contractor.

As mentioned before, the remediation plan and cost estimates were prepared assuming that only primary grouting points will be needed. We anticipate that a more detailed grouting plan, including bid documents, technical specifications and engineer's cost estimate, will be prepared in the next phase of the project.



9.0 <u>REPORT LIMITATIONS</u>

The conclusions and recommendations presented in this report assume that site conditions are not substantially different than those encountered by the explorations. If during construction, subsurface conditions are observed or appear to be different from those encountered in the explorations, Wood should be advised promptly so that those conditions can be reviewed, and recommendations reevaluated, where necessary.

The boring logs represent the subsurface conditions at the specific location at the time of the exploration. The subsurface conditions at other locations or at different times may differ, and no warranty as to the subsurface conditions elsewhere or at different times is expressed or implied by the data presented herein. Furthermore, the depths on the boring logs designating the interface between the various soils and rocks may only be approximate boundaries where the transition is gradual or could not be detected by the boring operations. In addition, the depth of the groundwater table, if encountered, is only indicative of the conditions at the time of the borings as groundwater level may fluctuate significantly because of various factors.

The recommendations provided in this report are based on the scope of the exploration and testing program. In addition, this report does not reflect the subsurface conditions below the tested depths.

The evaluation of conditions that may be encountered in construction requires engineering judgment and interpretation. For this reason, we recommend that Wood remain involved with this project during the construction process, particularly during grouting operations. If we are not retained during construction, we cannot assume responsibility for misinterpretation of our recommendations, or for unfavorable foundation performance as a result of judgments rendered by others.



10.0 <u>REFERENCES</u>

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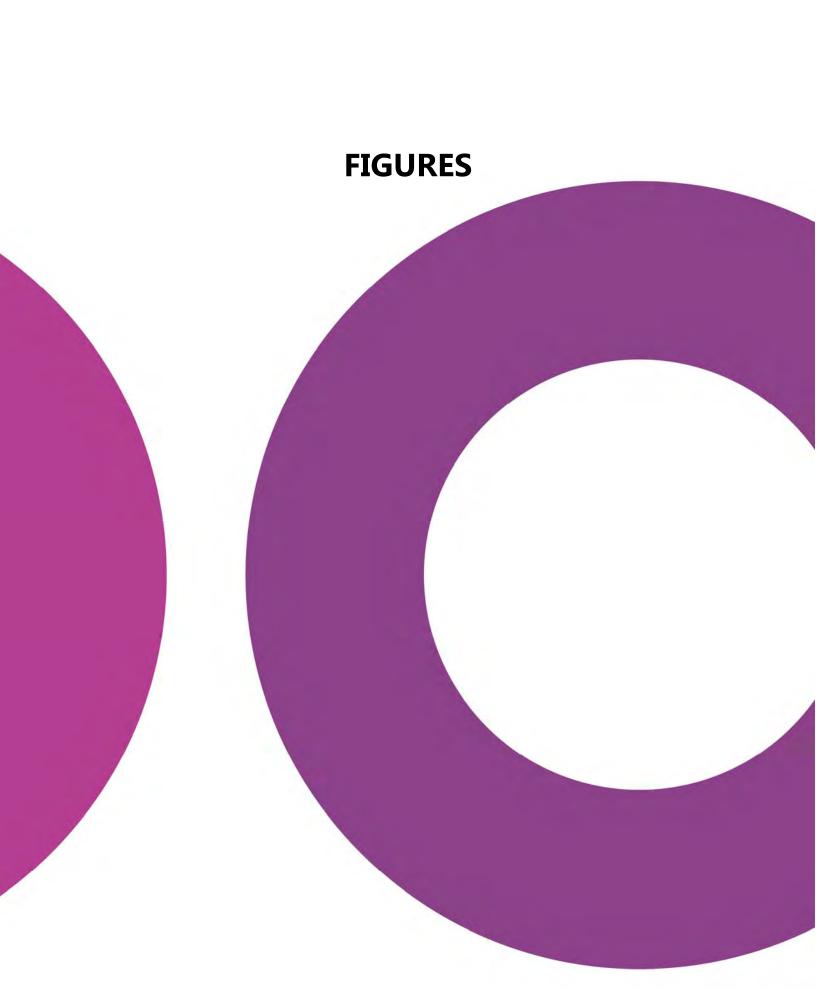
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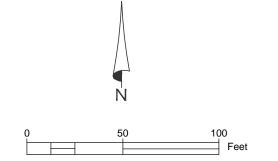
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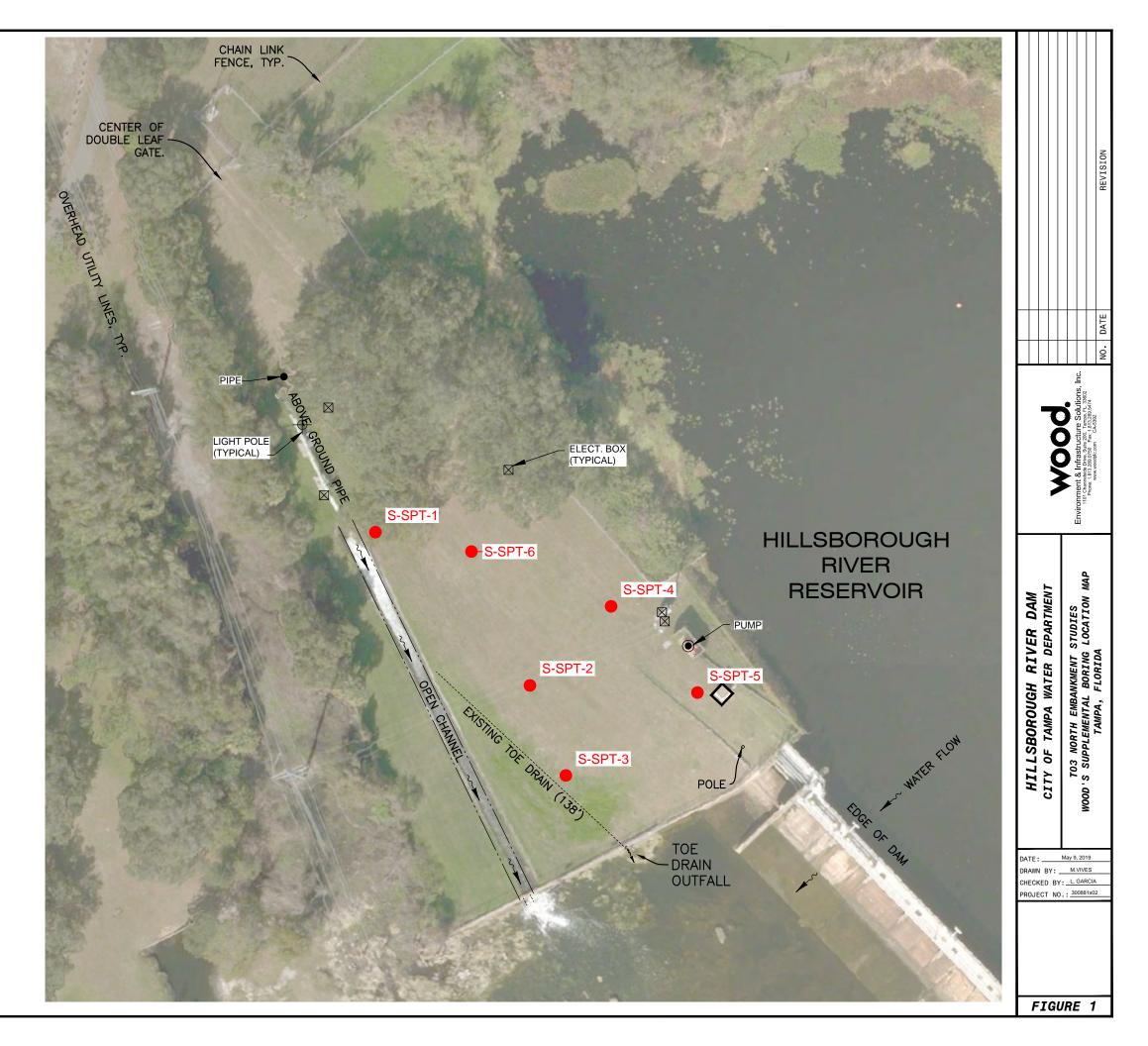


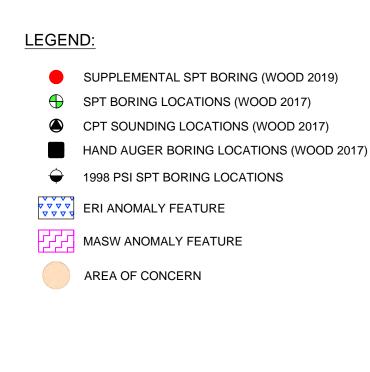


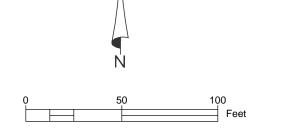


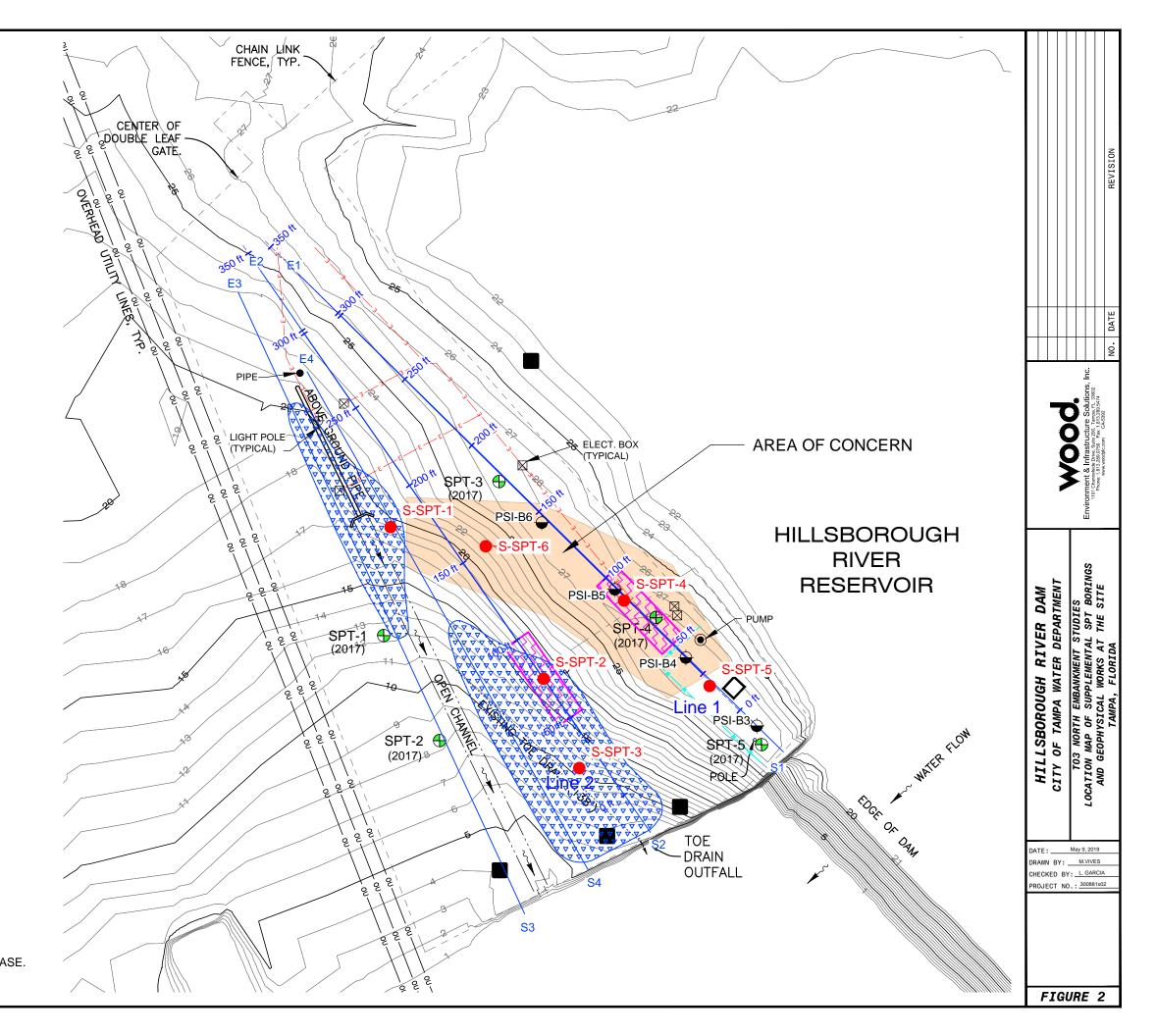


SUPPLEMENTAL SPT BORING (WOOD 2019)

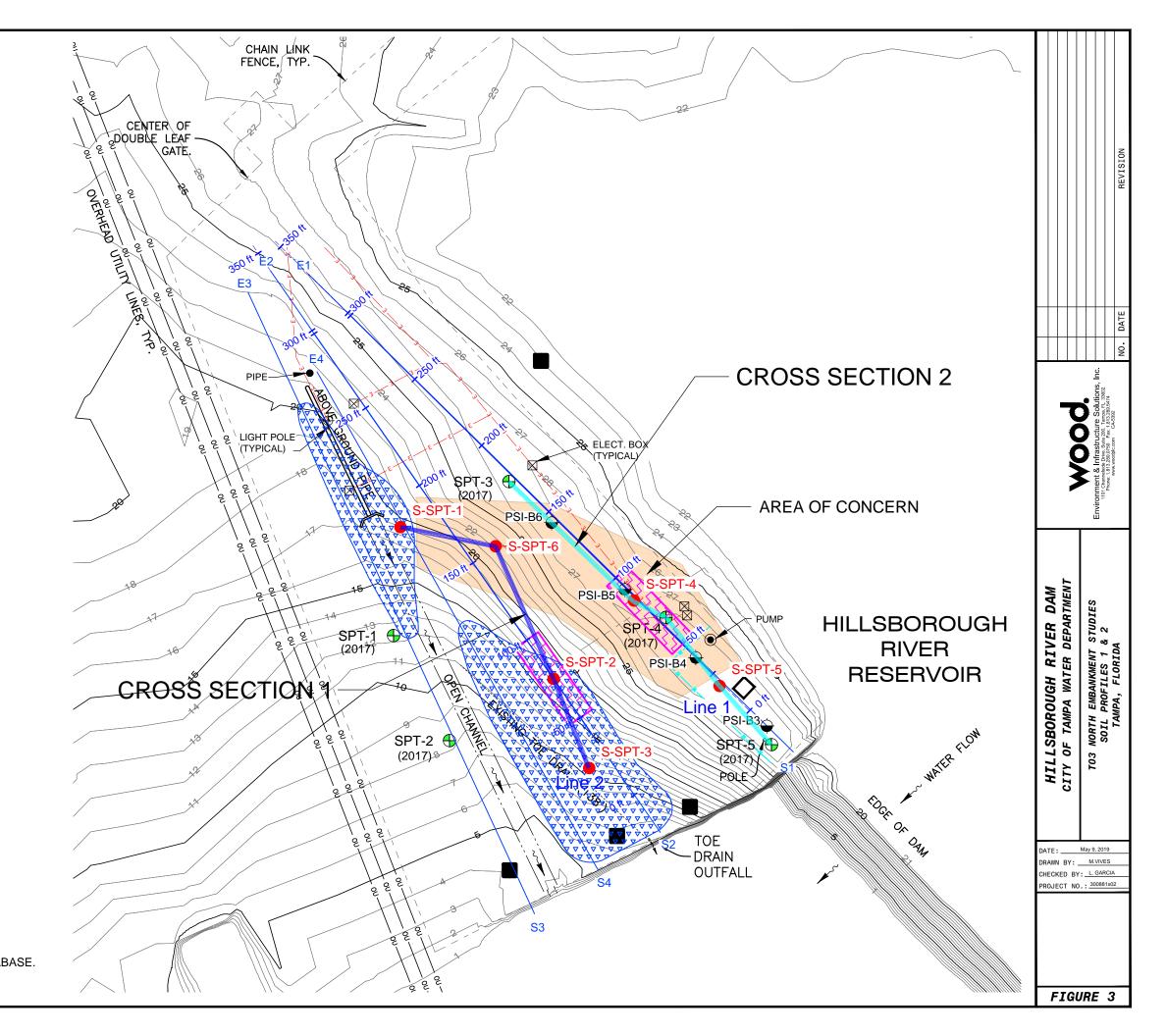


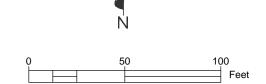


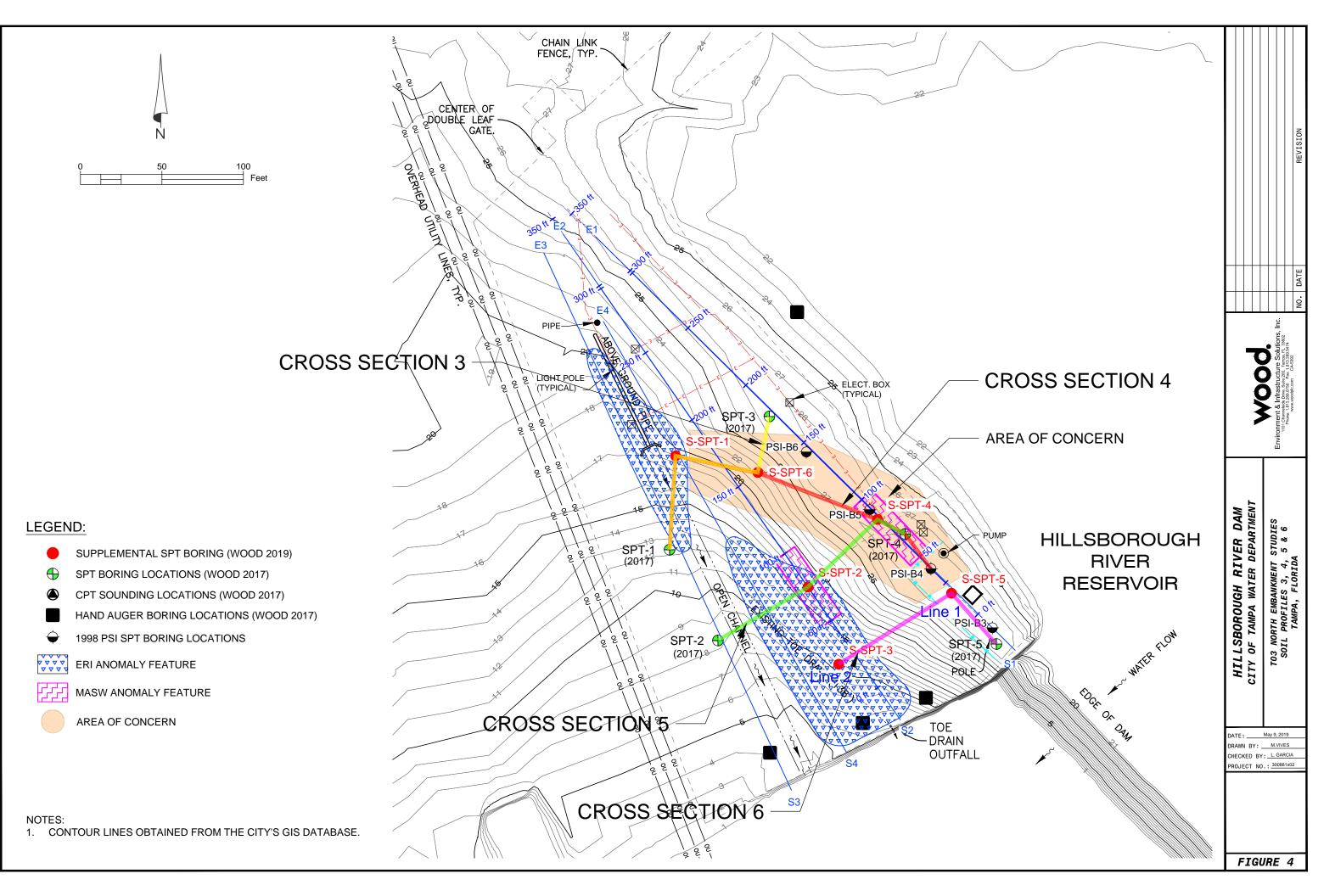


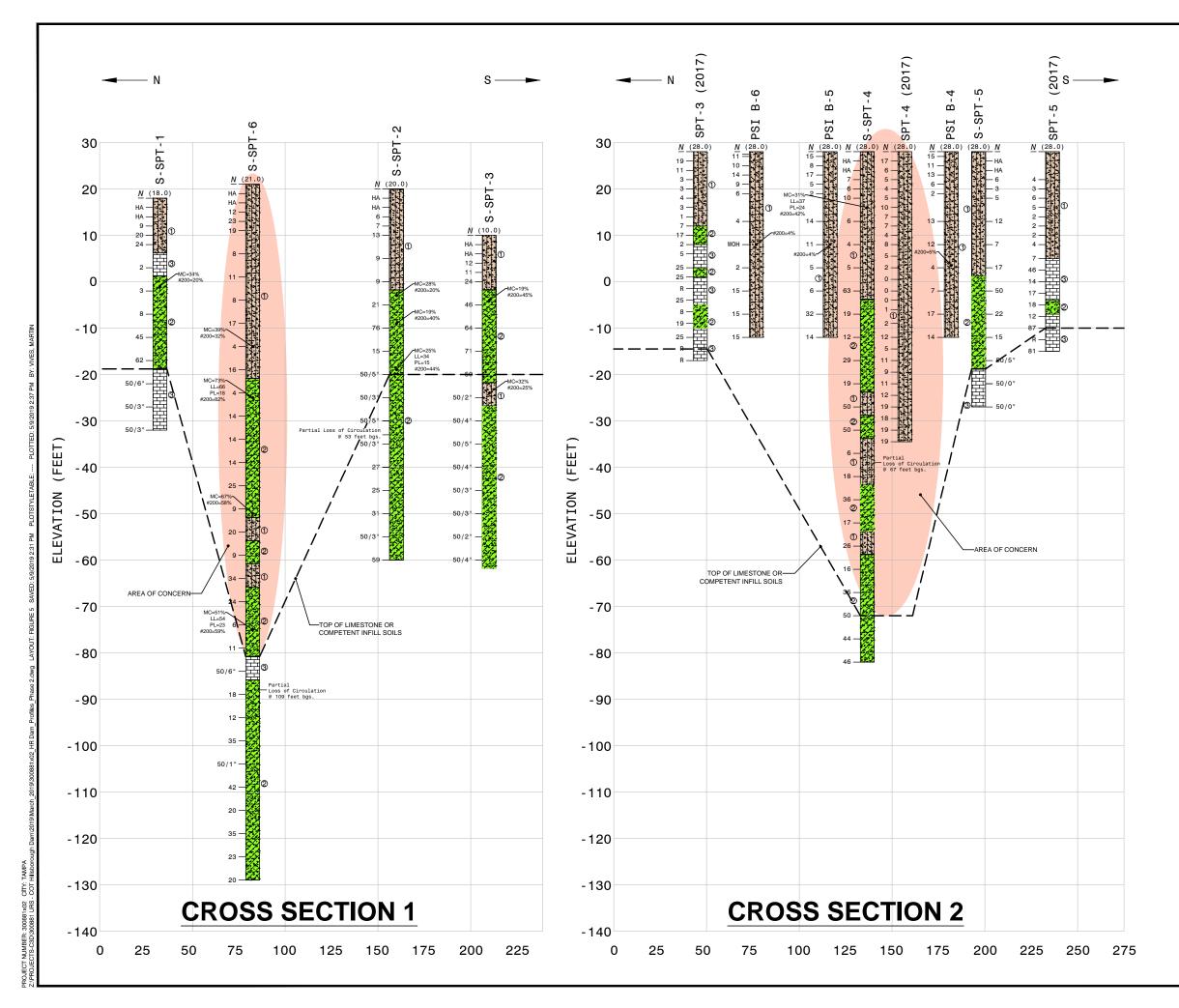






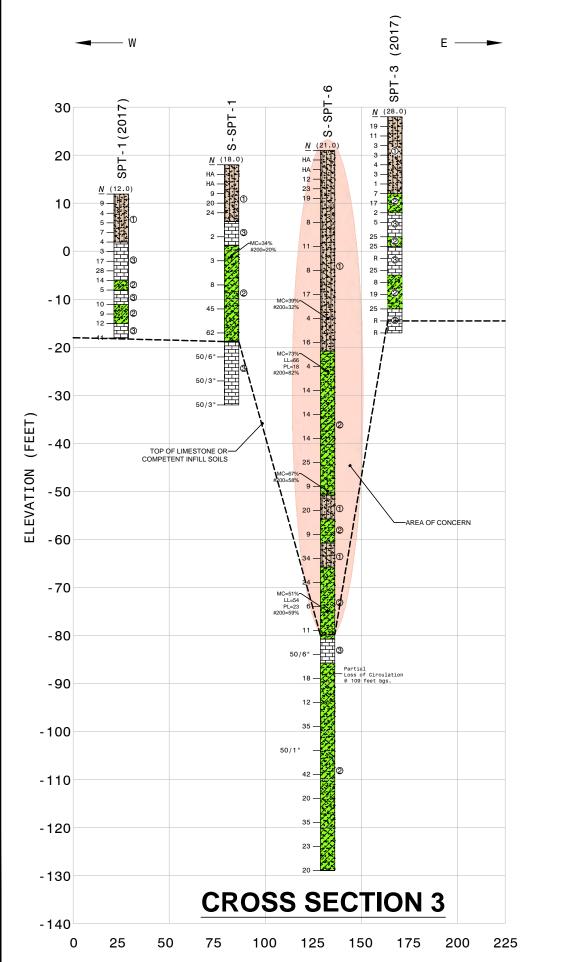


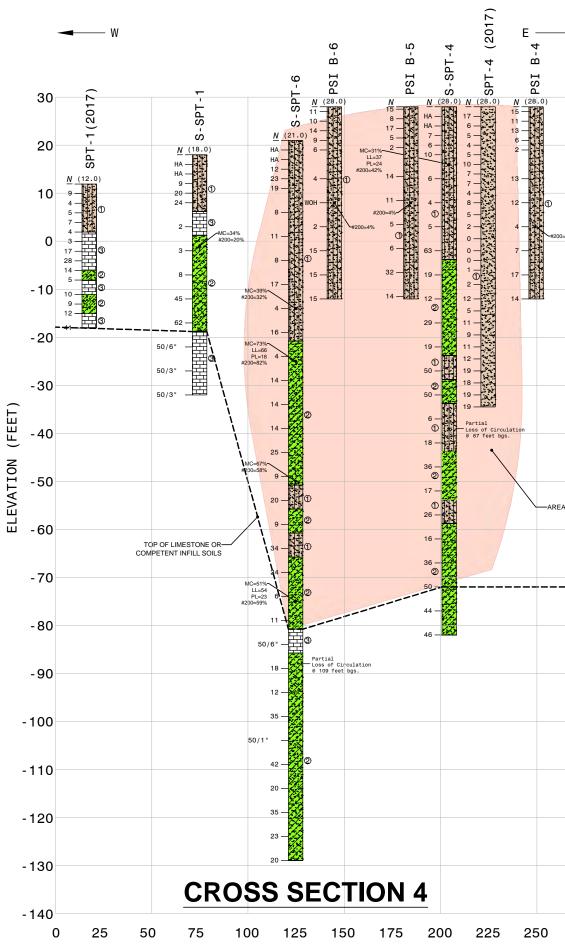




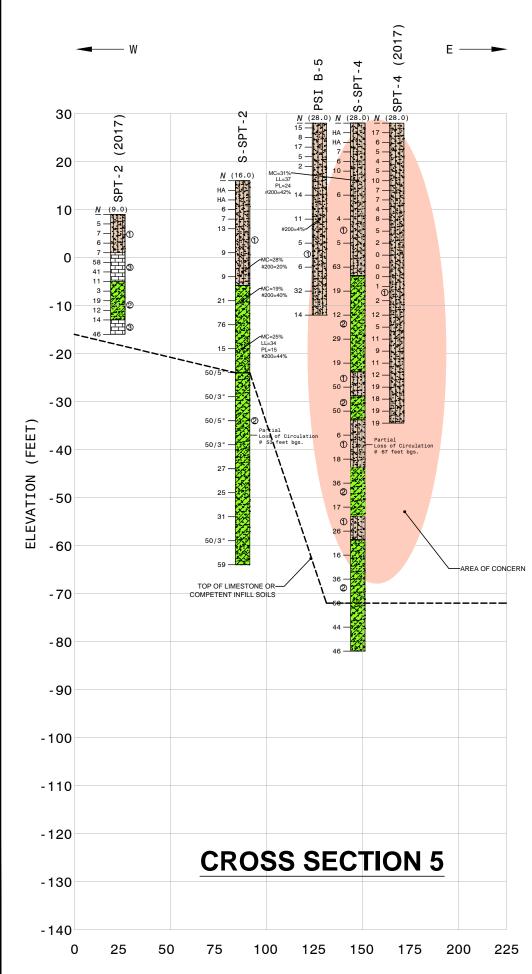
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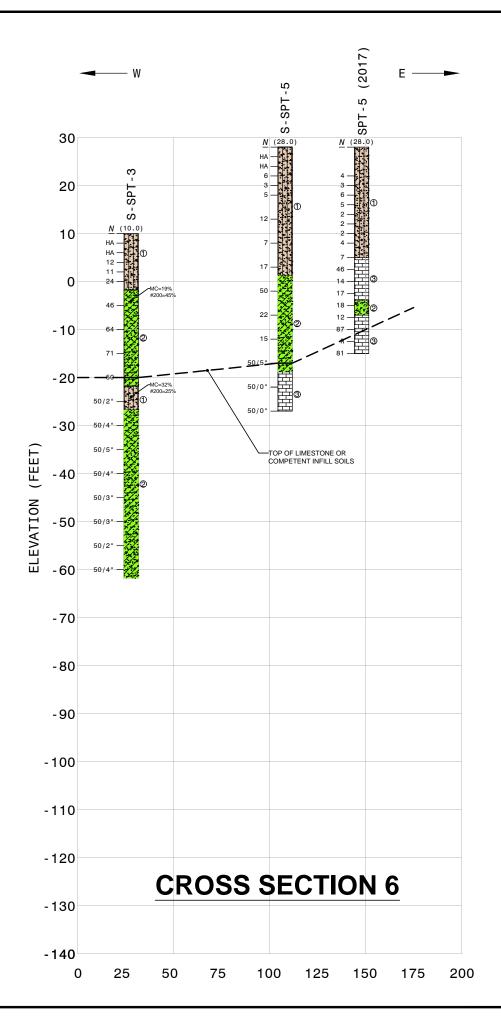
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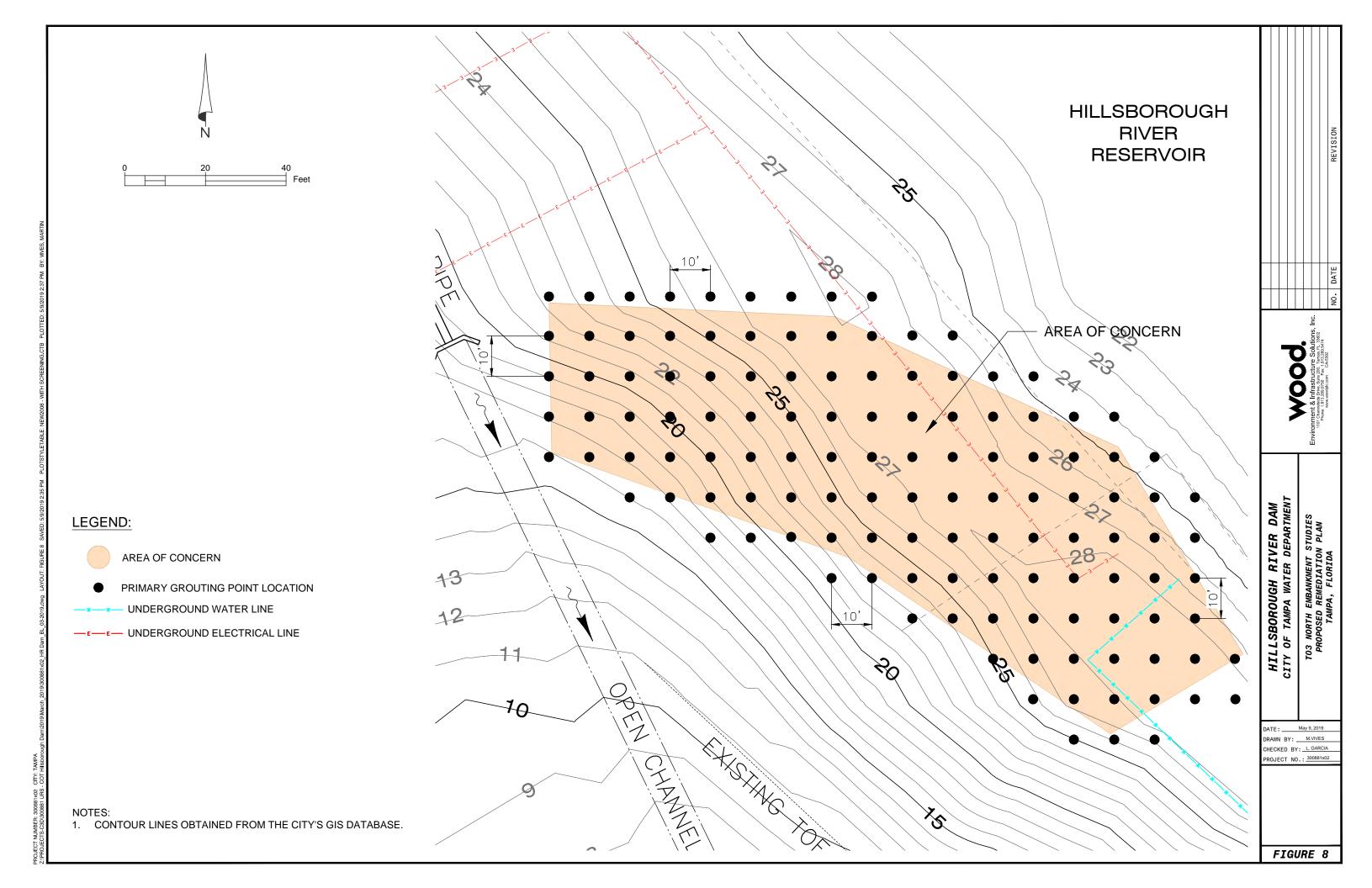
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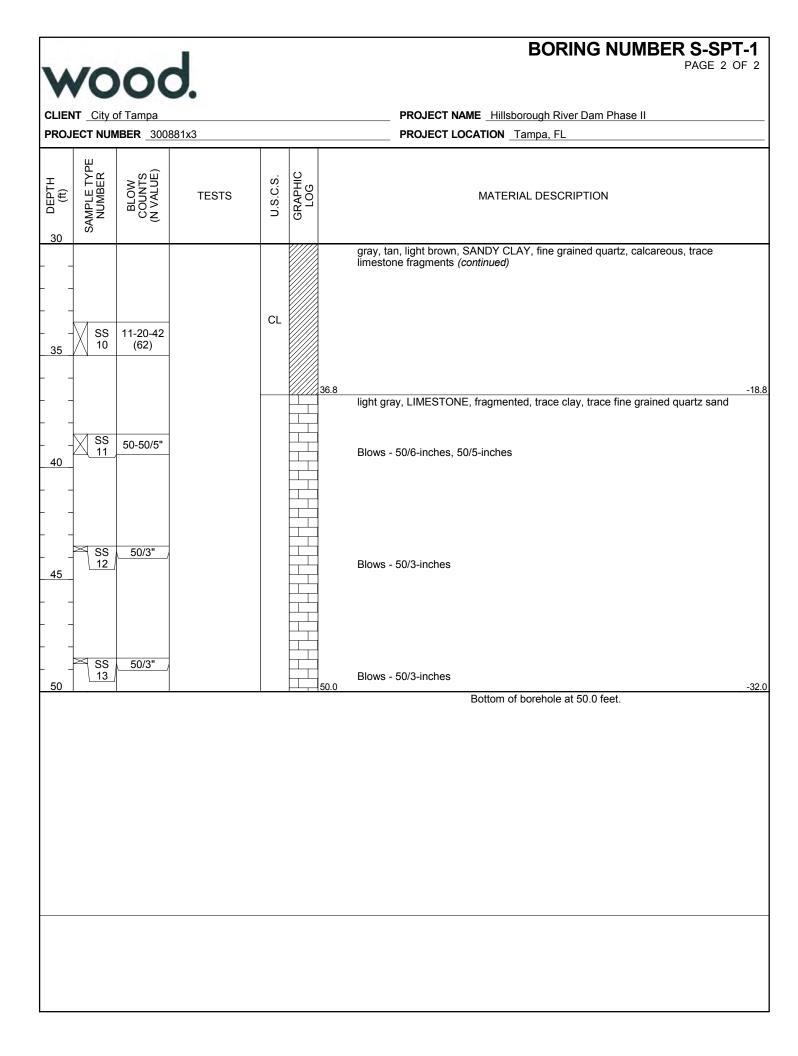
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FIGURE 7



APPENDIX A

V		000	J .			BORING NUMBER S-SPT-1 PAGE 1 OF 2
		of Tampa				PROJECT NAME Hillsborough River Dam Phase II
		JMBER 300				PROJECT LOCATION Tampa, FL
DATE	START	ED 2/11/19	СОМ	PLETE	D 2/	/12/19 GROUND ELEVATION 18 ft HOLE SIZE 2.75 inches
			Madrid Engineer			
			ndard Penetration /			
		CS				MC HOLE COMPLETION Tremie grout to surface
			f the embankment			
o DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	AL			0.14		brown, gray, dark gray, SILTY SAND, fine grained quartz, trace gravel, trace clay, trace roots, moist
	AL 2			SM		4.0 ▽ 14
	SS 3	1-5-4-7 (9)		SP- SC		gray, brown, poorly sorted, fine grained quartz SAND with CLAY, wet
	$\left(\right)$				Y/	dark gray, gray, brown, poorly sorted, fine grained quartz SAND, trace clay,
		10-11-9-11 (20)				trace limestone fragments, wet
 _ <u>10</u> 	SS 5	13-18-6-5 (24)		SP		Drilling fluid used below 10-feet
						light gray, brown, LIMESTONE, fragmented, trace clay, trace fine grained quartz sand
 _ 15		0-2-4 (6)				Blows - WOH/6", 2, 4
L _	-					
	-					16.8 gray, tan, light brown, CLAYEY SAND, fine grained quartz, calcareous, trace
	-					limestone fragments
	ss 7	0-0-3 (3)	MC = 34% #200 = 29%	-		Blows - WOH/12", 3
	-			sc		
	 ∕∕ ss	3-4-4				
25	8	(8)				
	-					26.8 -6
				CL		gray, tan, light brown, SANDY CLAY, fine grained quartz, calcareous, trace limestone fragments
 30	ss 9	30-28-17 (45)				



BORING NUMBER S-SPT-2 PAGE 1 OF 3 CLIENT City of Tampa PROJECT NAME Hillsborough River Dam Phase II PROJECT NUMBER 300881x3 PROJECT LOCATION _Tampa, FL DATE STARTED 2/14/19 **COMPLETED** <u>2/14/19</u> GROUND ELEVATION 16 ft HOLE SIZE 2.75 inches DRILLING CONTRACTOR Madrid Engineering Group LOCATION N1342053.3, E517695.5 ▽ GROUND WATER LEVEL AT TIME OF DRILLING 6.00 ft / Elev 10.00 ft DRILLING METHOD Standard Penetration / Mud Rotary LOGGED BY CS CHECKED BY MC HOLE COMPLETION Tremie grout to surface NOTES _Located on mid slope of the embankment SAMPLE TYPE NUMBER BLOW COUNTS (N VALUE) GRAPHIC LOG U.S.C.S. DEPTH (ft) TESTS MATERIAL DESCRIPTION 0 light brown, poorly sorted, fine grained quartz SAND, trace silt, moist AU 1 SP AU 2 12.0 4.0 light brown, gray, poorly sorted, fine grained guartz SAND with CLAY, with silt, SS 1-2-4-4 5 moist 3 (6) ∇ wet below 6-feet SS 1-2-5-9 4 (7) angular shell and oyster fragments from 6 to 10 feet SP SC SS 9-8-5-5 5 (13) 10 drilling fluid used below 10-feet 11.8 4.3 gray, light gray, poorly sorted, fine grained quartz SAND, with limestone fragments, trace shell fragments SS 4-5-4 SP 6 (9) 15 16.8 -0.8 light brown, CLAYEY SAND, fine grained quartz, calcareous, with limestone fragments SS 7-5-4 MC = 28% #200 = 20% 7 (9) 20 SC SS 6-11-10 MC = 19% 8 #200 = 40% (21) 25 26.8 -10.8 light gray, gray, light brown, blue-gray, SANDY CLAY, calcareous, with limestone fragments CL SS 14-26-50 9 (76)30

M	/0	00	J .			BORING NUMBER S-SPT-2 PAGE 2 OF 3
	IT <u>City c</u> ECT NUN	of Tampa	881x3			PROJECT NAME _ Hillsborough River Dam Phase II PROJECT LOCATION _ Tampa, FL
DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
30 -				CL		light gray, gray, light brown, blue-gray, SANDY CLAY, calcareous, with limestone fragments <i>(continued)</i>
-						31.8 -1 light gray, gray, light brown, CLAYEY SAND, fine grained quartz, calcareous, with limestone fragments
- <u>35</u>	SS 10	8-5-10 (15)	MC = 25% #200 = 44% LL = 34 PL = 15 PI = 19	sc		
-						36.8 -2 light gray, gray, light brown, blue-gray, SANDY CLAY, calcareous, with limestone fragments
_ 40	⊠ SS 11	50/5"				Blows - 50/5-inches
- - <u>45</u>	SS 12	7-50/3"				Blows - 7, 50/3-inches
- - <u>50</u>	SS 13	39-50/5"		CL		Blows - 39, 50/5-inches
_ 55	SS 14	1-5-50/3"				partial loss of circulation and shell fragments from 53 to 55 feet Blows - 1, 5, 50/3-inches
- - 60	SS 15	9-12-15 (27)				

woo	d.		BORING NUMBER S-SPT-2 PAGE 3 OF 3
CLIENT City of Tampa			PROJECT NAME _Hillsborough River Dam Phase II
PROJECT NUMBER _30	0881x3		PROJECT LOCATION Tampa, FL
B DEPTH (ft) (ft) SAMPLE TYPE NUMBER BLOW COUNTS (N VALUE)	TESTS	GRAPHIC LOG	MATERIAL DESCRIPTION
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$			light gray, ight brown, blue-gray, SANDY CLAY, calcareous, with limestone fragments (continued) partial loss of circulation below 73.5 feet Blows - WOR/6-inches, 26, 50/3-inches
			Bottom of borehole at 80.0 feet.

		00	4			BORING NUMBER S-SPT-3 PAGE 1 OF 3
CLIEI PROJ DATE	NT <u>City o</u> IECT NUM STARTE	of Tampa //BER _ 300a ED _ 2/12/19	881x3 COM	PLETE	D <u>2/</u>	PROJECT NAME Hillsborough River Dam Phase II PROJECT LOCATION Tampa, FL 14/19 GROUND ELEVATION 10 ft HOLE SIZE 2.75 inches LOCATION N1342006 E517714.4
			idard Penetration /			
LOGO	GED BY _	CS	CHEC	CKED	BY M	C HOLE COMPLETION Tremie grout to surface
NOTE	S Locat	ed on mid s	lope of the emban	kment	1	
o DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	AU					gray, light gray, light brown, poorly sorted, fine grained quartz SAND, moist
	AU 2	-		SP		wet below 2 feet
5	ss 🛛	1-1-11-9				
	3	(12)				6.0 4.0 gray, dark gray, poorly sorted, fine grained quartz SAND with SILT, trace
	ss 4	2-3-8-10 (11)		SP- SM		limestone fragments, wet 8.0 2.0
		8-7-17-14				gray, poorly sorted, fine grained quartz SAND, trace silt, trace limestone fragments, wet
	5	(24)		SP		drilling fluid used below 10-feet
	-			 		11.8 -1.8 light gray, CLAYEY SAND, fine grained quartz, calcareous, with limestone fragments
15	SS 6	19-16-30 (46)	MC = 19% #200 = 45%	sc		
	_					
						16.8 -6.8 light gray, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments
	SS 7	22-14- 50/5"				Blows - 22, 14, 50/5-inches
	-					
				CL		
25	SS 8	21-21- 50/5"				Blows - 21, 21, 50/5-inches
 	-					
30	SS 9	31-23-27 (50)				

(Continued Next Page)

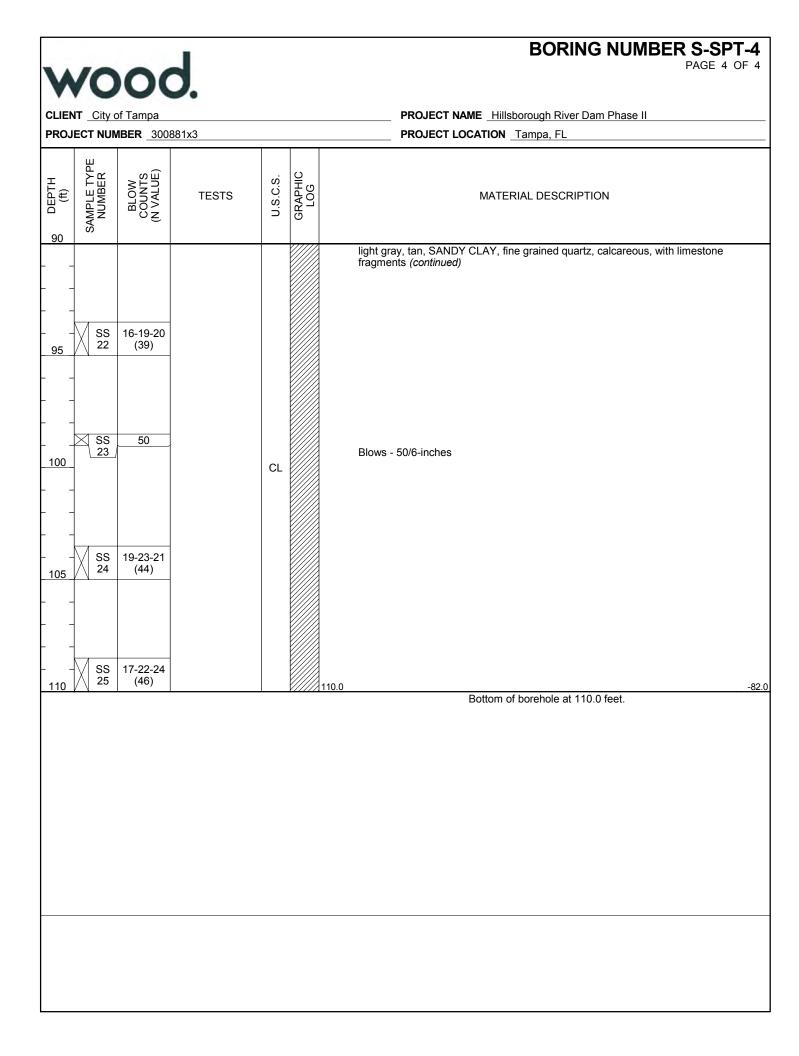
N	/0	00	J .			BORING NUMBER S-SPT-3 PAGE 2 OF 3
LIEN	T City o	of Tampa				PROJECT NAME _ Hillsborough River Dam Phase II PROJECT LOCATION Tampa, FL
(1) 30	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
- - 35	SS 10	17-50/2"	MC = 32% #200 = 25%	CL		light gray, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments <i>(continued)</i> 31.8 -2 greenish-gray, CLAYEY SAND, fine grained quartz Blows - 17, 50/2-inches
	SS 11	50/4"				36.8 -2 light gray, gray, light brown, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments Blows - 50/4-inches
- - 15 -	⊠ SS 12	50/5"				Blows - 50/5-inches
- - 50 _	SS 13	19-50/4"		CL		Blows - 19, 50/4-inches
- - 55 -	SS 14					blue-gray to light brown, non-calacareous from 48.5 to 55 feet Blows - 50/3-inches
- - - 30	SS 15	28-35- 50/3"				Blows - 28, 35, 50/3-inches

BORING NUMBER S-SPT-3 PAGE 3 OF 3 100 CLIENT City of Tampa PROJECT NAME Hillsborough River Dam Phase II PROJECT NUMBER _300881x3 PROJECT LOCATION _Tampa, FL SAMPLE TYPE NUMBER BLOW COUNTS (N VALUE) GRAPHIC LOG DEPTH (ft) U.S.C.S. TESTS MATERIAL DESCRIPTION 60 light gray, gray, light brown, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments *(continued)* SS 16 50/2" Blows - 50/2-inches 65 CL SS 50/4" 17 Blows - 50/4-inches 70 -60.0 70.0 Bottom of borehole at 70.0 feet.

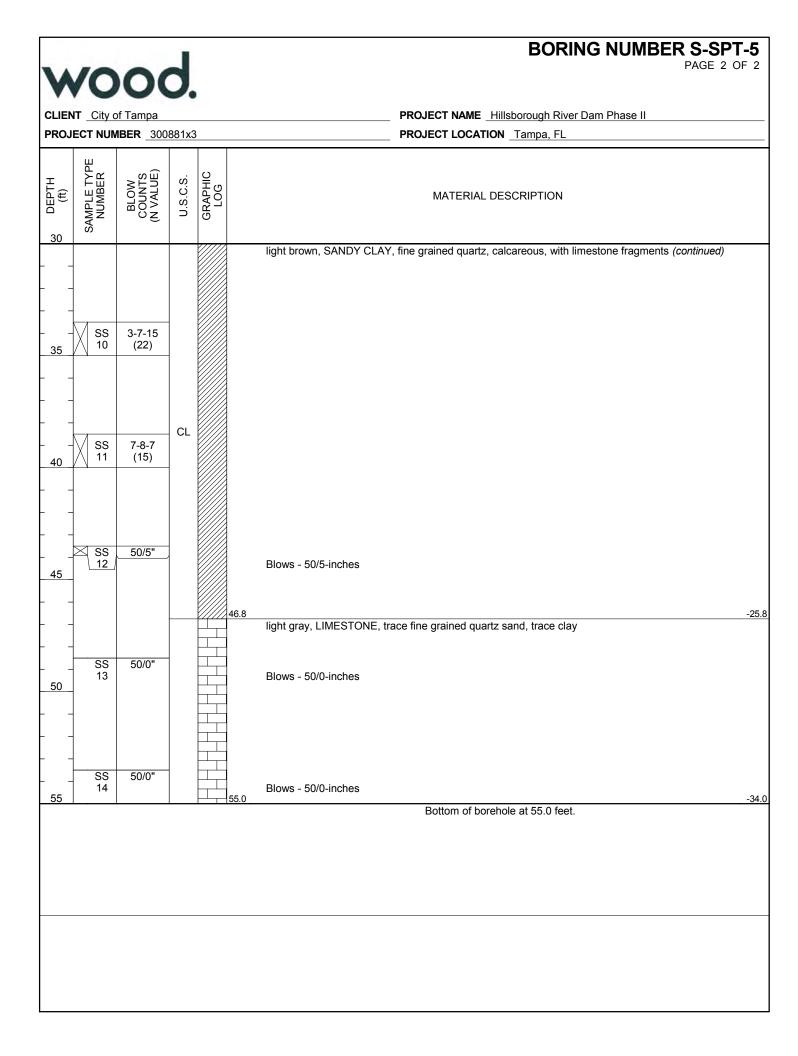
	3.1		1			BORING NUMBER S-SPT-4 PAGE 1 OF 4
V	/C	00) .			
		of Tampa				PROJECT NAME Hillsborough River Dam Phase II
			881x3			PROJECT LOCATION _Tampa, FL
DATE	START	ED _2/20/19	COM	PLETE	D _2/2	20/19 GROUND ELEVATION _28 ft HOLE SIZE _2.75 inches
DRILI	ING CO	NTRACTOR	Madrid Engineeri	ng Gro	oup	LOCATION N1342094.8 , E517738.2
				Mud F	Rotary	GROUND WATER LEVEL AT TIME OF DRILLING
					BY M	C HOLE COMPLETION Tremie grout to surface
NOTE	S Loca	ted on crest	of the embankmer	nt	1	
o DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	AU 1 AU 2	-		SP- SM		brown, light brown, poorly sorted, fine grained quartz SAND with SILT, with clay nodules, moist 4.0 24.0
5	∭ ss	3-4-3-4				light brown, orange-brown, light gray, poorly sorted, fine grained quartz SAND, trace silt, moist
	3	(7)				trace gravel from 0 to 6 feet
	🛛 ss	4-3-3-5				
	4	(6)				
	🛛 ss	3-5-5-5				
10	5	(10)				
						drilling fluid used below 10-feet
	-					
	-					
	SS SS	4-2-4				
_ 15 _	6	(6)				
	-					
L -	-			SP		
	∬ ss	3-2-2				trace limestrone fragments from 18.5 to 20 feet
20	7	(4)				, o
	-					
	-					
	SS SS	3-1-4				trace organics from 23.5 to 25 feet
_ 25	8	(5)				
	-					
ļ .	-					
L.						
	∬ ss					limestone fragments and trace calcareous clay from 28.5 to 30 feet
30	9	6-13-50/5"				Blows - 6, 13, 50/5-inches

M	/0	00	J.			BORING NUMBER S-SPT PAGE 2 O	
		of Tampa				PROJECT NAME Hillsborough River Dam Phase II	
ROJ		MBER _ 3008	381x3			PROJECT LOCATION _ Tampa, FL	
(#) 30	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
_				SP		light brown, orange-brown, light gray, poorly sorted, fine grained quartz SAND, trace silt, moist <i>(continued)</i> 31.8	-
- - - - - -	SS 10	7-9-10 (19)		SC		light brown, light gray, CLAYEY SAND, fine grained quartz, calcareous, trace limestone fragments	
- 0 -	SS 11	6-4-8 (12)	MC = 31% #200 = 42% LL = 37 PL = 24 PI = 13			41.8	_
_ _ 5	SS 12	28-14-15 (29)				light brown, light gray, SANDY CLAY, fine grained quartz, calcareous, trace limestone fragments	
- - 0	SS 13	23-13-6 (19)		CL		51.8	_
_ _ 5_	⊠ SS 14	50/5"		sc		light gray, greenish gray, CLAYEY SAND, fine grained quartz, trace limestone fragments Blows - 50/5-inches	
- - - 0	≍ SS 15	50/5"		CL		56.8 light gray, SANDY CLAY, fine grained quartz, calcareous, trace limestone fragments Blows - 50/5-inches	

wood.	BORING NUMBER S-SPT-4 PAGE 3 OF 4
CLIENT _City of Tampa PROJECT NUMBER _300881x3	PROJECT NAME _ Hillsborough River Dam Phase II PROJECT LOCATION _ Tampa, FL
DEPTH (ft) (ft) (ft) BLOW COUNTS (N VALUE) (N VALUE)	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	light gray, SANDY CLAY, fine grained quartz, calcareous, trace limestone fragments (continued) 61.8 -33 brown, gray, poorly sorted, fine grained quartz SAND, trace silt
	partial loss of circulation below 67-feet
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	light gray, dark gray, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments
80 - SS 6-8-9 19 (17)	81.8 -53 dark gray, poorly sorted, fine grained quartz SAND, trace silt, slightly calcareous, with limestone
SS 10-11-15 85 (26)	5 86.8 -58
90 SS 3-7-9 21 (16)	light gray, tan, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments



4						BORING NUMBER S-SPT-5
VA	10	00				PAGE 1 OF 2
		of Tampa IBER _300	001./2	,		
					COMPLETED <u>2/21/19</u>	PROJECT LOCATIONTampa, FL GROUND ELEVATION _21 ft HOLE SIZE _2.75 inches
						LOCATION N1342049.5 , E517783.6
					ration / Mud Rotary	
						HOLE COMPLETION Tremie grout to surface
NOTES	Locat	ed on mid s	slope	of the e	embankment	
o DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION
	AU 1		SP- SC			grained quartz SAND with CLAY, with angular shell fragments, moist
	AU				2.0 brown, light orange-brown	, light gray, poorly sorted, fine grained quartz SAND, trace silt, moist
	2					
	SS 3	5-4-2-2 (6)				
		2-1-2-1 (3)				
	ss 5	3-2-3-2 (5)				
	V J	(3)			drilling fluid used below 10)-feet
15	SS 6	3-4-8 (12)	SP			
20	SS 7	3-4-3 (7)				
	•					
	∕ ss	9-10-7			trace organics and trace o	lay from 23.5 to 25 feet
25/	8	(17)				
F 1					26.8	-5.8
F 1					light brown, SANDY CLAY	/, fine grained quartz, calcareous, with limestone fragments
			CL			
30	SS 9	12-23-27 (50)				
/	N			*/////	1	



woo				BORING NUMBER S-SPT-6 PAGE 1 OF 5
CLIENT <u>City of Ta</u> PROJECT NUMBE DATE STARTED _ DRILLING CONTR DRILLING METHO LOGGED BY <u>CS</u>	ampa R 300881x3 2/15/19 COM ACTOR Madrid Engineer D Standard Penetration	PLETEI ing Gro / Mud R CKED B	oup totary	PROJECT LOCATIONTampa, FL GROUND ELEVATION21 ft HOLE SIZE2.75 inches LOCATION N1342123.7 , E517664.8
	TESTS		GRAPHIC LOG	MATERIAL DESCRIPTION
AU 1 AU 2 5 SS 5- 3 5 SS 5-1 4 5 SS 6-6	6-6-8 (12) 1-12-12 (23) -13-14 (19)	SP- SC	drill 11.8	t brown, gray, poorly sorted, fine grained quartz SAND with CLAY, moist ing fluid used below 10-feet
6 SS€	3-4-4 (8) 3-5-6 (11)	SP	fraç 21.8	t brown, dark gray, poorly sorted, fine grained quartz SAND, trace limestone ments
25 8 	2-3-5 (8) 3-8-9 (17)	sc		estone fragments and shells present from 23.5 to 25 feet

CLIEN	T City o	Df Tampa				BORING NUMBER S-SPT-6 PAGE 2 OF 5 PROJECT NAME _ Hillsborough River Dam Phase II PROJECT LOCATION _ Tampa, FL
6 DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
 35	SS 10	3-1-3 (4)	MC = 39% #200 = 32%	sc		light gray, dark gray, CLAYEY SAND, fine grained quartz <i>(continued)</i>
 - 40	SS 11	7-7-9 (16)				limestone fragments present from 38.5 to 40 feet 41.8 -20.8
 <u>45</u> 	SS 12	2-2-2 (4)	MC = 73% #200 = 82% LL = 66 PL = 18 PI = 48			gray, dark gray, blue-gray, SANDY CLAY, fine grained quartz, fatty
 <u>50</u> 	SS 13	5-7-7 (14)		CL		
 _ <u>55</u> 	SS 14	6-8-6 (14)				
60	SS 15	6-7-7 (14)				

		O C C	J .			BORING NUMBER S-SPT-6 PAGE 3 OF 5 PROJECT NAME Hillsborough River Dam Phase II
PROJ	IECT NUM	IBER _3008	881x3	1	1	PROJECT LOCATION Tampa, FL
9 DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
 <u>65</u>	- SS 16	7-10-15 (25)		CL		gray, dark gray, blue-gray, SANDY CLAY, fine grained quartz, fatty <i>(continued)</i>
	SS 17	3-3-6 (9)	MC = 67% #200 = 58%	_		71.8 -50
 _ 75	SS 18	7-8-12 (20)		SP		light gray, poorly sorted, fine grained quartz SAND, trace clay, with limestone fragments
 - 80	- SS 19	3-4-5 (9)		CL		-55 light gray, blue-gray, SANDY CLAY, fine grained quartz, slightly calcareous
85	SS 20	8-15-19 (34)		SP		81.8 -60 light gray, poorly sorted, fine grained quartz SAND, trace clay, with limestone fragments
 90	SS 21	5-10-14 (24)		CL		86.8 -65. dark gray, brown, blue-gray, SANDY CLAY, fine grained quartz, fatty

WO	00	J.			BORING NUMBER S-SPT-6 PAGE 4 OF 5
CLIENT <u>City</u>	of Tampa				PROJECT NAME _ Hillsborough River Dam Phase II PROJECT LOCATION _ Tampa, FL
6 DEPTH (ft) SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
90 90 5 95 95 105 105 105 105 105 105 105 10	3-3-3 (6) 6-6-5 (11) 50 7-8-10 (18) 7-8-10 (18) 5-5-7 (12) 14-13-22 (35)	MC = 51% #200 = 59% LL = 54 PL = 23 PI = 31	CL		dark gray, brown, blue-gray, SANDY CLAY, fine grained quartz, fatty (continued) 101.8 101.8 101.8 101.8 101.8 101.8 102.8 103.8 104.8 105.8 106.8 106.8 106.8 106.8 106.8 106.8 106.8 106.8 106.8 107. 108.8 108.8 109.8

wood	1		BORING NUMBER S-SPT-6 PAGE 5 OF 5
CLIENT _City of Tampa PROJECT NUMBER _3008			PROJECT NAME _Hillsborough River Dam Phase II PROJECT LOCATION _Tampa, FL
BEPTH (ft) (ft) (ft) (ft) (ft) (ft) (ft) (ft)	TESTS	U.S.C.S. GRAPHIC	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		CL	light gray, SANDY CLAY, fine grained quartz, calcareous, with limestone fragments (continued) Blows - 28, 50/1-inch

							BORING NUMBER	
		10		d			P	AGE 1 OF 1
	\wedge			U.				
CLIE	NT .	City o	of Tampa				PROJECT NAME _ Hillsborough River Dam	
PRO.	JEC		IBER 3008	881x2			PROJECT LOCATION _ Tampa, FL	
DATE	E ST	ARTE	D <u>11/8/17</u>	COM	PLETE	D <u>11</u>	/8/17 GROUND ELEVATION HOLE SIZE _4 ir	nches
DRIL	LING	g con	TRACTOR	Madrid Engineeri	ng Gr	oup	LOCATION	
			HOD Mud				GROUND WATER LEVEL AT TIME OF DRILLING	
LOG	GED	BY _	CS	CHEC	KED	BY M	C HOLE COMPLETION backfilled with cuttings and bentonite ch	ips
NOTE	ES _	W 82	.42947 deg	; N 28.02507 deg				
o DEPTH (ft)		SAMPLE IYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
	\mathbb{N}	SS 1	2-4-5-4 (9)		sc		dark gray, brown, CLAEY SAND, fine grain quartz, gravel and shell frag moist 2.0	gment present,
	\mathbb{N}	SS 2	3-1-3-4 (4)	MC = 21% #200 = 10%	SP-		brown, gray, fine grain quartz SAND with Clayey, shell fragments prese	nt, wet
5	\mathbb{N}	SS 3	2-2-3-2 (5)		SC		6.0	
	\mathbb{N}	SS 4	1-5-2-4 (7)		SP- SC		dark gray, gray, SAND with Clayey, fine grain quartz, wet	
F .	$\overline{\mathbb{N}}$	SS	3-3-1-3	MC = 24%	SP-	T ÍÍ	gray, dark gray, fine grain quartz SAND with SILT	
10	1Å	5	(4)	#200 = 9%	SM		clean sand lense in bottom 6" of spoon 10.0	
_ 10	\uparrow						very light brown, light brown LIMESTONE, with light brown calcareous of	clay present
		SS 6	3-1-2-10 (3)					
		SS 7	7-3-14-15 (17)					
15	-	SS 8	4-9-19-16 (28)					
F .	$\overline{\mathbb{N}}$	SS	17-7-7-16					
- ·	1X	9	(14)					
		SS 10	3-2-3-3 (5)	MC = 28% #200 = 50% LL = 36 PL = 14	CL		 18.0 light brown, light orange-brown, Calcareous CLAY with traces of limesto grain quartz sand present 20.0 	
				PI = 22	1		very light brown, light brown, LIMESTONE with light brown calcareous of	clay present
F .	\mathbb{N}	SS	4-5-5					
		11	(10)				22.5 gray, light gray, Calcareous CLAY, with traces of gray limestone preser	nt
Ļ.		SS	3-4-5	MC = 34%	-			
_ 25	Å	12	(9)	#200 = 64%	CL			
	+							
	-	SS 13	4-3-9 (12)				27.5 light brown, light gray, LIMESTONE, with an abundance of light brown of	alcaroous
		SS	14-17-24				clay present	Calcaleous
30	Ŵ	14	(41)				30.0	

Bottom of borehole at 30.0 feet.

		00	d.				BORING NUMBER SPT-2 PAGE 1 OF 1
		of Tampa					PROJECT NAME Hillsborough River Dam
PROJ	ECT N	JMBER _ 300					PROJECT LOCATION _Tampa, FL
							GROUND ELEVATION HOLE SIZE 4 inches
							GROUND WATER LEVEL AT TIME OF DRILLING
			I Rotary				HOLE COMPLETION backfilled with cuttings and bentonite chips
			g; N 28.02498 deg				
	ш						
DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION
0	V ss	2-2-3-5					dark gray, gray, CLAYEY SAND, fine grain quartz, moist, traces of shell fragments
				SC		2.0	
			MC = 22% #200 = 13%				gray, light gray, brown, CLAYEY SAND, fine grain quartz, moist, traces of shell fragments
5	SS 3			sc			
			MC = 25% #200 = 13%			8.0	limestone found in bottom 2 inches of spoon
 10	SS 5						very light brown, light brown, LIMESTONE, with light brown calcareous clay
		27-14-27- 16 (41)					
	S8 7	8 8-6-5-9 (11)				14.0	
15							light brown, gray, light gray, calcareous CLAY, with sand and abundance of limestone orange-brown material found from 14 - 16 feet deep
		5 7-12-7-10 (19)	MC = 23% #200 = 36%	_			
 20	St 10	5 5-3-9-9 (12)	MC = 25% #200 = 44% LL = 44 PL = 19 PI = 25	CL			
		(14)				22.5	gray, light gray, LIMESTONE with light gray calcareous clay
	V se						
25	12	(46)				25.0	Bottom of borehole at 25.0 feet.

		/ C		d.				E	BORING NUMBER SPT-3 PAGE 1 OF 2
			of Tampa					PROJECT NAME Hillsborough	River Dam
			IBER 300					PROJECT LOCATION Tampa,	
									HOLE SIZE 4 inches
				Madrid Engineer					
1								\bigtriangledown GROUND WATER LEVEL AT	TIME OF DRILLING 8.00 ft
1								HOLE COMPLETION backfilled	
		_		; N 28.02539 deg					· · ·
o DEPTH (ft)		NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC I OG			DESCRIPTION
[.	М	SS	7-10-9-7				brown, l	ght brown, SAND with Silty, fine g	grain quartz, gravel fragments, dry
L.	\square	1	(19)		SP-				
	\mathbb{N}	SS 2	4-5-6-5 (11)		SM		4.0		
5		SS 3	2-2-1-1 (3)					wn, brown, fine grain quartz SANI	D with SILT, moist
	\mathbb{N}	SS 4	1-1-2-1 (3)	MC = 6% #200 = 9%	SP- SM				
10	\mathbb{N}	SS 5	1-2-2-3 (4)				wet belo	w 8 feet deep	
		SS 6	1-2-1-3 (3)		SP- SM			y, gray, fine grain quartz SAND w	ith Silty, wet
		SS 7	2-0-1-1 (1)		SP- SM		brown, l	ght brow, fine grain quartz SAND	with SILT
_ 15		SS 8	2-2-5-8 (7)	MC = 19% #200 = 22%	SC			ht brown, CLAYEY SAND, fine gr	ain quartz, with silt
	$\left\{ \right\}$						16.0 grav. ve	llowish brown, SANDY CLAY, trac	ces of limestone
		SS 9	6-8-9-8 (17)		CL		3.2,,,,	·····, · · · · · · · , · · · · , · · · · · , · · · · · , · · · · · · , · · · · · · · · · , ·	
20	\mathbb{N}	SS 10	1-1-1-2 (2)				20.0		
 	\mathbb{N}	SS 11	0-0-5-9 (5)				very ligh	t brown, light brown, LIMESTONE	E, with very light brown calcareous clay
							1		
- ·		SS	6-6-19						
_ 25	М	12	(25)				25.0	thrown Colographic CLAV with	an abundance of yory light brown limesters
		SS 13	12-13-12 (25)		CL		27.5		an abundance of very light brown limestone
	-							t brown, LIMESTONE, with very I	ight brown calcareous clay
30		SS 14	29-50						

BORING NUMBER SPT-3

CLIENT City of Tampa PROJECT NAME Hillsborough River Dam PROJECT NUMBER _300881x2 PROJECT LOCATION _Tampa, FL SAMPLE TYPE NUMBER BLOW COUNTS (N VALUE) GRAPHIC LOG DEPTH (ft) U.S.C.S. TESTS MATERIAL DESCRIPTION 30 very light brown, LIMESTONE, with very light brown calcareous clay (continued) SS 13-9-16 15 (25) 33.5 greenish gray, Calcareous CLAY, abundance of limestone SS MC = 25% 5-4-4 16 (8) #200 = 42% 35 SS 7-8-11 CL 17 (19) SS 6-6-19 18 (25) 40 40.0 very light brown, grayish brown, LIMESTONE, with very light brown calcareous clay SS 50 19 SS 50 20 45 145.0 Bottom of borehole at 45.0 feet.

							BORING NUMBER SPT-4 PAGE 1 OF 3
	VC		O.				
CLIER	NT _City o	of Tampa					PROJECT NAME Hillsborough River Dam
							PROJECT LOCATION Tampa, FL
							GROUND ELEVATION HOLE SIZE 4 inches LOCATION HOLE SIZE 4 inches
			Rotary				
							HOLE COMPLETION backfilled with cuttings and bentonite chips
			; N 28.02521 deg		DI		
o DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC I OG		MATERIAL DESCRIPTION
	ss 1	8-11-6-5 (17)		SP- SM			t brown, light brown, fine grain quartz SAND with Silty, dry
	ss 2	2-3-3-3		SM		2.0 light brov	wn, brown, SILTY SAND, moist, traces of angular gravel
Ļ.	<u> </u>	(6)				4.0	
5	ss 3	2-3-2-2 (5)	MC = 4% #200 = 6%			light bro	wn, brown, fine grain quartz SAND with Silty, moist
	ss 4	2-2-2-2 (4)				light ora	nge-brown material from 4 - 6 feet deep
	ss 5	2-2-3-4 (5)					
<u>10</u>	SS 6	3-3-7-5 (10)		SP-			
	SS 7	3-3-4-4 (7)		SM		light ora	nge-brown material from 12 - 14 feet deep
_ 15	SS 8	1-2-5-5 (7)					
 	SS 9	2-2-2-3 (4)	MC = 25% #200 = 5%			light gra	/ material from 16 - 20 feet deep
20	SS 10	3-5-3-4 (8)				20.0	
L.	<u> </u>					light gra	y, gray, fine grain quartz SAND SAND with Silty
	ss 11	1-1-4 (5)					
25	SS 12	0-1-1 (2)		SP-			
	SS 13	0-0-0 (0)		SM			
 	SS 14	1-0-0-0 (0)					

V	VC		d.			BORING NUMBER SPT-4 PAGE 2 OF 3
	IT <u>City c</u>	of Tampa IBER _3008	881x2			PROJECT NAME _ Hillsborough River Dam PROJECT LOCATION _ Tampa, FL
05 DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	SS 15	0-0-0-1 (0)				light gray, gray, fine grain quartz SAND SAND with Silty <i>(continued)</i> dark gray material found 30 - 32 feet deep
	SS 16	0-1-0-1 (1)		SP- SM		
35	SS 17	0-0-2-7 (2)				light brown material found 34 -36 feet deep 36.5
						light brown, brown, fine grain quartz SAND SAND with Silty
40	SS 18	5-6-6 (12)				
	SS 19	3-3-2 (5)				
45	SS 20	2-5-6 (11)		SP- SM		
	SS 21	3-4-5 (9)				
- – - – 50	SS 22	3-4-7 (11)				50.0
	SS 23	3-3-9 (12)	MC = 31% #200 = 22%	SM		gray, dark gray, SILTY SAND, fine grain quartz, dark gray clayey sand present
55	SS 24	4-8-11 (19)				gray, light brown, fine grain quartz SAND SAND with Silty, traces of phosphatic sand present
	SS 25	5-8-10 (18)		SP- SM		
 60	SS 26	4-8-11 (19)				

BORING NUMBER SPT-4 PAGE 3 OF 3 NOOC CLIENT City of Tampa PROJECT NAME Hillsborough River Dam PROJECT NUMBER _300881x2 PROJECT LOCATION _Tampa, FL SAMPLE TYPE NUMBER BLOW COUNTS (N VALUE) GRAPHIC LOG DEPTH (ft) U.S.C.S. TESTS MATERIAL DESCRIPTION 60 gray, light brown, fine grain quartz SAND SAND with Silty, traces of phosphatic sand present *(continued)* Dark gray material found beneath 61 feet BGS SP-SS 27 5-9-10 SM (19) 62 5 Bottom of borehole at 62.5 feet.

						BORING NUMBER SPT-5
						PAGE 1 OF 2
		of Tampa				PROJECT NAME Hillsborough River Dam
			881x2			PROJECT LOCATION _Tampa, FL /7/17 GROUND ELEVATION HOLE SIZE _4 inches
	DRILLING METHOD Mud Rotary					
						IC HOLE COMPLETION backfilled with cuttings and bentonite chips
NOTE	S <u>W 82</u>	.42887 deg	; N 28.02499 deg			
DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	SAM	" ^Ŭ Z			5	
	AU 1					brown, light brown, fine grain quartz SAND with Silty, dry, angular gravel fragments
	AU 2			SP-		
	SS 3	3-2-2-1 (4)	MC = 8% #200 = 10%	_		dry until 6 feet deep, then moist below
	SS 4	1-1-2-1 (3)				8.0
	ss 5	3-3-3-3 (6)				brown, light brown, fine grain quartz, SAND with Silty traces of angular gravel fragments
	SS 6	2-2-3-2 (5)				reddish-brown gravel present from 8 - 10 feet deep
	SS 7	0-0-2-2 (2)	MC = 24% #200 = 5%	SP- SM		
15	SS 8	1-1-1-2 (2)				
	SS 9	1-1-1-2 (2)				18.0
20	SS 10	1-2-2-5 (4)		0.0		gray, light brown, fine grain quartz SAND with Silty
	∕∕ ss	2-3-4		SP- SM		
	11	(7)				22.5 light brown, gray, LIMESTONE, with an abundance of brown calcareous clay
	SS 12	8-29-17 (46)				
 	SS 13	8-8-6 (14)				27.5 very light brown, LIMESTONE, with very light brown calcareous clay
30	SS 14	5-9-8 (17)				

BORING NUMBER SPT-5

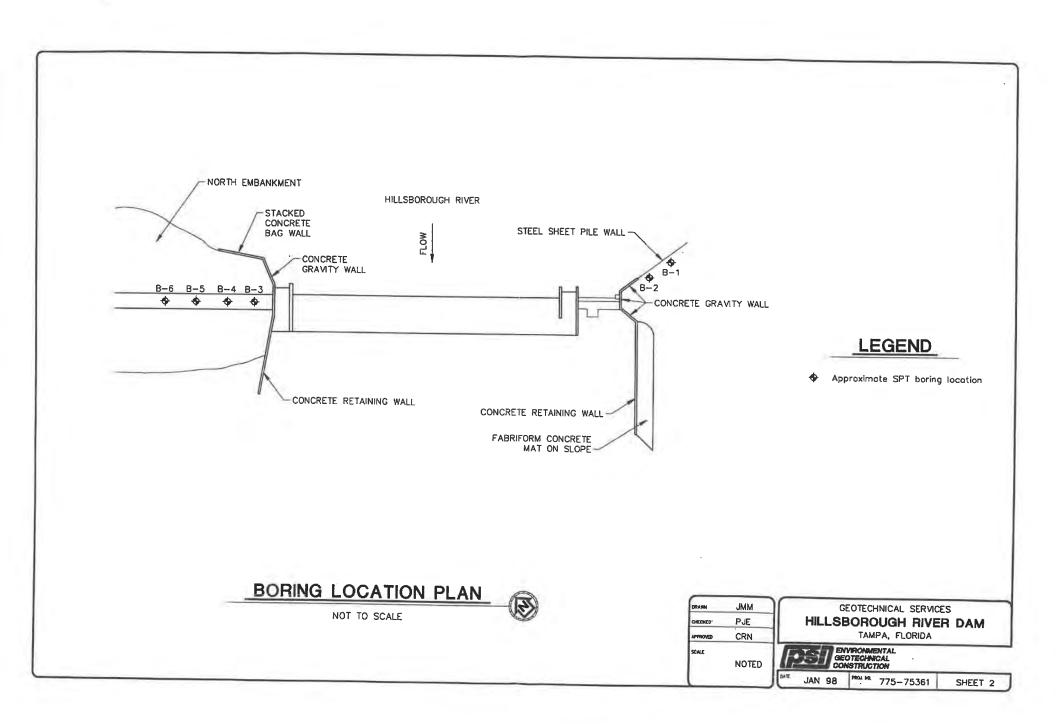
wood.

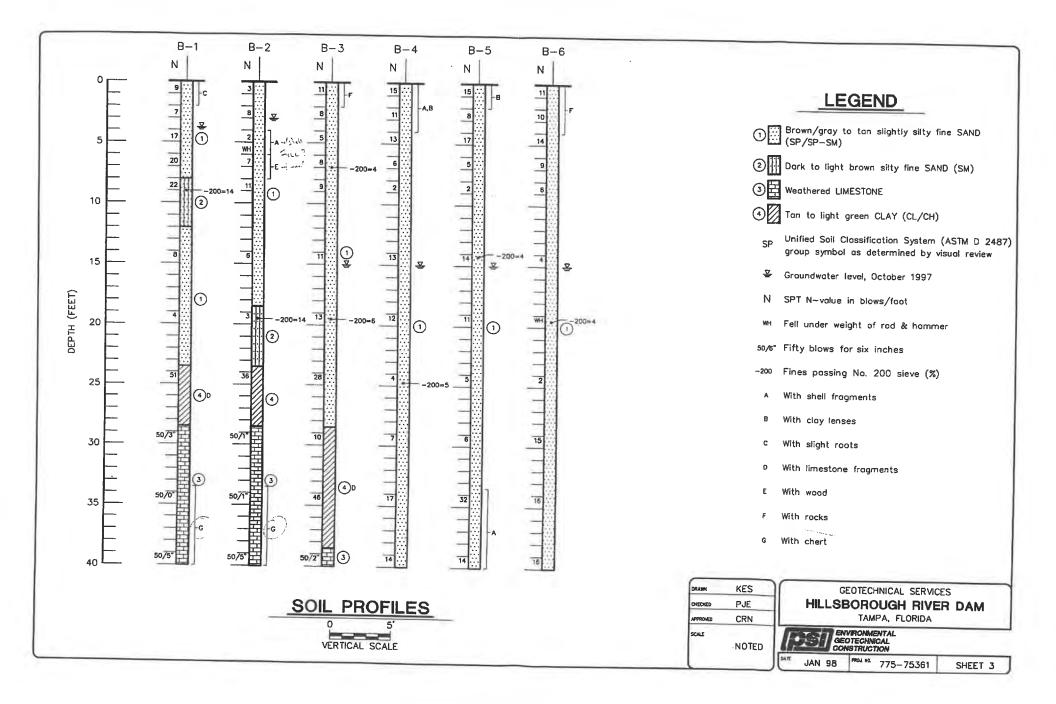
CLIENT City of Tampa

PROJECT NAME Hillsborough River Dam

PROJ	ROJECT NUMBER _ 300881x2 PROJECT LOCATION _ Tampa, FL					
00 DEPTH (ft)	SAMPLE TYPE NUMBER	BLOW COUNTS (N VALUE)	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
	SS 15	15-10-8 (18)				very light brown, LIMESTONE, with very light brown calcareous clay <i>(continued)</i> 32.5
	SS 16	2-3-9 (12)	MC = 37% #200 = 61%	CL		very light brown, Calcareous CLAY, with limestone fragments, dark gray material in bottom 6" of spoon
<u>35</u>	SS 17	11-37-50 (87)				 very light brown, LIMESTONE, with abundance of light brown calcareous clay in top 12" of spoon Abundance of light brown calcareous clay from 36 - 37 feet BGS
 _ 40	SS 18	50				
	SS 19	27-31-50 (81)				42.5 Bottom of borehole at 42.5 feet.

APPENDIX B





APPENDIX C



Boring ID	Depth (ft)	Moisture Content (%) [ASTM- D2216]	% Finer #200 Sieve (%) [ASTM-D1140]	Liquid Limit	Plastic Limit	Plasticity Index
SPT-1	18.5'-20.0'	33.8	29.2	-	-	NP
SPT-2	18.5'-20.0'	28.0	20.0	-	-	-
SPT-2	23.5'-25.0'	18.5	40.3	-	-	-
SPT-2	33.5'-35.0'	31.9	24.8	34	15	19
SPT-3	13.5'-15.0'	19.3	44.6	-	-	NP
SPT-3	33.5'-35.0'	31.9	24.8	-	-	-
SPT-4	38.5'-40.0'	31.4	42.4	37	24	13
SPT-6	33.5'-35.0'	39.3	31.9	-	-	-
SPT-6	43.5'-45.0'	72.9	82.3	66	18	48
SPT-6	68.5'-70.0'	67.1	57.9	-	-	-
SPT-6	93.5'-95.0'	51.0	59.2	54	23	31

Laboratory Test Result Summary



MOISTURE CONTENT and WET SIEVE ANALYSIS

CLIENT: URS & City of Tampa Water Department

Address: 7650 W. Courtney Campbell Causeway
Suite 700

Tampa, Florida 33602-1462

Project: Hillsborough River Dam Phase II

Location: Tampa, Florida

	% Solids, Moisture Content							Wet Sieve Test		
Sample No. and Depth	Tare ID	Weight of Container (g)	Weight of Container + Wet Soil (g)	Weight of Container + Dry Soil (g)	Solids Content (%)	Moisture Content (%)	Weight of Container + Dry Soil (g)	Weight of Container + Dry washed Soil (g)	% Finer than #200 Sieve (%)	
SPT-1 18.5'-20'	1	8.39	231.04	174.80	74.74	33.8	174.80	126.28	29.2	
SPT-2 18.5'-20'	2	8.40	352.02	276.85	78.12	28.0	276.85	223.09	20.0	
SPT-2 23.5'-25'	3	8.39	258.75	219.66	84.39	18.5	219.66	134.50	40.3	
SPT-2 33.5'-35'	4	8.35	237.26	190.89	79.74	25.4	190.89	111.57	43.5	
SPT-3 13.5'-15'	5	8.35	302.40	254.91	83.85	19.3	254.91	145.05	44.6	
SPT-3 33.5'-35'	6	8.34	246.96	189.24	75.81	31.9	189.24	144.43	24.8	
SPT-4 38.5'-40'	7	8.35	209.36	161.36	76.12	31.4	161.36	96.41	42.4	
SPT-6 33.5'-35'	9	8.34	283.90	206.17	71.79	39.3	206.17	143.14	31.9	
SPT-6 43.5'-45'	10	8.33	213.36	126.92	57.84	72.9	126.92	29.37	82.3	
SPT-6 68.5'-70'	11	8.33	212.30	130.41	59.85	67.1	130.41	59.77	57.9	
SPT-6 93.5'-95'	12	8.29	215.15	145.30	66.23	51.0	145.30	64.21	59.2	

Test Date: March 13, 2019 Project #: 300881x3 Requested By: C. Suarez Tested By: M. Hall Checked By: C Suarez



Date: March 12, 2019

300881X3

C. Suarez

C. Suarez

M. Hall

SPT-1

18.5'-20'

7

CLIENT: URS & City of Tampa Water Department

7650 W. Courtney Campbell Causeway Address:

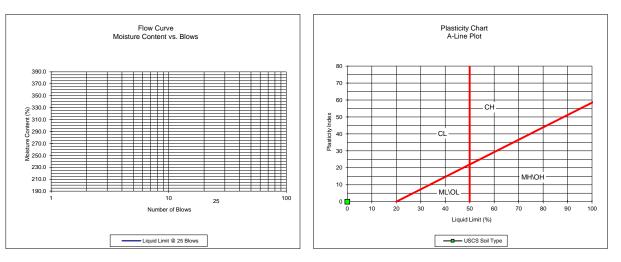
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II				
Location:	Tampa, Florida				

	Liquid Limit									
Number of	Weight of Cont.	Weight of Cont.	Weight of	Moisture						
Blows	+ Wet Soil	+ Dry Soil	Container	Content						
	(grams)	(grams)	(grams)	(%)						

Plastic Limit							
Weight of Cont.	Weight of Cont.	Weight of	Moisture				
+ Wet Soil	+ Dry Soil	Container	Content				
(grams)	(grams)	(grams)	(%)				



	Natural Moisture Content, Percent Passing #200 Sieve								
Cont+wet									
soil	Cont. + dry soil	Container wt.	Moisture Content	Cont+dry wash	% passing #200				
231.04	174.8	8.39	33.8%	126.28	29.2%				

NOTES:

Results Summary Liquid Limit: Plastic Limit: Plasticity Index

Project #:

Tested By:

Checked By:

Boring #:

Sample #:

Depth:

Requested By:

In-situ Moist. Content: 33.8%

% Passing #200: 29.2%

29.2%

NP



Date: March 12, 2019

Project #:

Tested By:

Checked By:

Boring #:

Sample #:

Depth:

Requested By:

300881X3

C. Suarez

C. Suarez

M. Hall

SPT-2

33.5-35

10

CLIENT: URS & City of Tampa Water Department

Address: 7650 W. Courtney Campbell Causeway

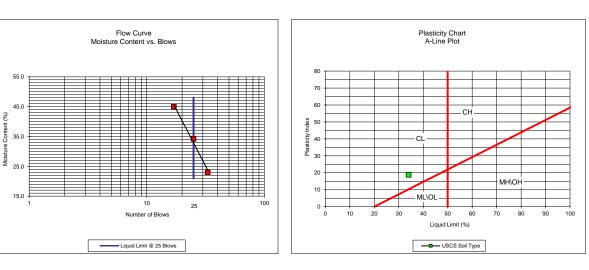
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II
Location:	Tampa, Florida

Liquid Limit									
Number of	Weight of Cont.	Weight of Cont.	Weight of	Moisture					
Blows	+ Wet Soil	+ Dry Soil	Container	Content					
	(grams)	(grams)	(grams)	(%)					
33	23.51	22.79	19.65	22.9					
25	11.51	10.75	8.53	34.2					
17	10.76	10.08	8.57	45.0					

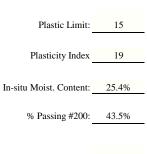
Plastic Limit							
Weight of Cont.	Weight of Cont.	Weight of	Moisture				
+ Wet Soil	+ Dry Soil	Container	Content				
(grams)	(grams)	(grams)	(%)				
15.96	14.97	8.56	15.4				
14.52	13.75	8.60	15.0				



	Natural Moisture Content, Percent Passing #200 Sieve								
Cont+we soil		Container wt.	Moisture Content	Cont+drv wash	% massing #200				
son	Cont. + dry soil	Container wt.	Moisture Coment	Cont+ury wash	% passing #200				
237.26	5 190.89	8.39	25.4%	111.57	43.5%				

NOTES:

Results Summary Liquid Limit: <u>34</u>



43.5%



Date: March 12, 2019

Project #:

Tested By:

Checked By:

Boring #:

Sample #:

Depth:

Requested By:

300881X3

C. Suarez

M. Hall

SPT-3

13.5'-15'

6

C. Suarez

CLIENT: URS & City of Tampa Water Department

7650 W. Courtney Campbell Causeway Address:

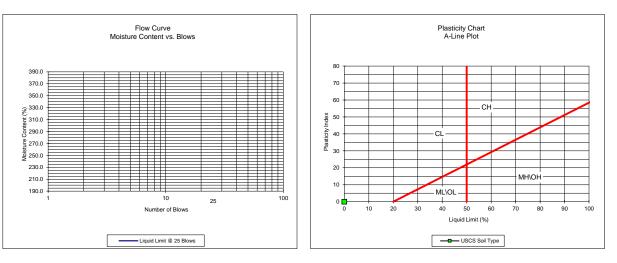
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II			
Location:	Tampa, Florida			

	Liquid Limit								
Number of	Weight of Cont.	Weight of Cont.	Weight of	Moisture					
Blows	+ Wet Soil	+ Dry Soil	Container	Content					
	(grams)	(grams)	(grams)	(%)					

Plastic Limit							
Weight of Cont. Weight of Cont. Weight of Moisture							
+ Wet Soil	+ Dry Soil	Container	Content				
(grams)	(grams)	(grams)	(%)				



	Natural Moisture Content, Percent Passing #200 Sieve						
	Cont+wet soil Cont. + dry soil Container wt. Moisture Content Cont+dry wash % passing #200						
L	SOII	Cont. + dry soil	Container wt.	Moisture Content	Cont+dry wash	% passing #200	
	231.04	174.8	8.39	33.8%	126.28	29.2%	

NOTES:

Results Summary Liquid Limit: Plastic Limit: Plasticity Index In-situ Moist. Content: 33.8%

% Passing #200: 29.2%

29.2%

NP



Date: March 12, 2019

300881X3

C. Suarez

C. Suarez

M. Hall

SPT-4

CLIENT: URS & City of Tampa Water Department

Address: 7650 W. Courtney Campbell Causeway

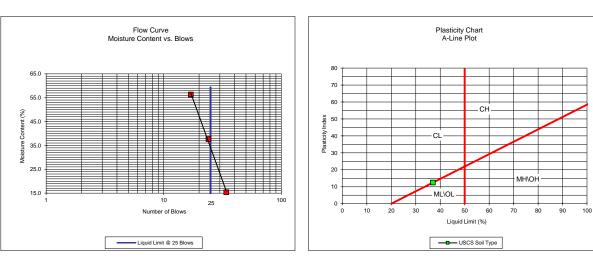
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II
Location:	Tampa, Florida

	Liquid Limit								
Number of	mber of Weight of Cont. Weight of Cont. Weight of Moisture								
Blows	+ Wet Soil	+ Dry Soil	Container	Content					
	(grams)	(grams)	(grams)	(%)					
34	10.39	10.15	8.59	15.4					
24	11.01	10.37	8.67	37.6					
17	11.85	10.65	8.52	56.3					

Plastic Limit							
Weight of Cont. Weight of Cont. Weight of Moisture							
+ Wet Soil	+ Dry Soil	Container	Content				
(grams)	(grams)	(grams)	(%)				
15.59	14.25	8.56	23.6				
16.78	15.12	8.59	25.4				



	Natural Moisture Content, Percent Passing #200 Sieve							
Cont+wet								
soil	Cont. + dry soil	Container wt.	Moisture Content	Cont+dry wash	% passing #200			
209.32	161.36	8.35	31.3%	96.41	42.4%			

NOTES:

Sample #: 11 Depth: 38.5-40 Results Summary Liquid Limit: 37

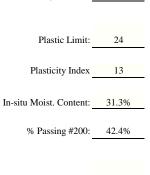
Boring #:

Project #:

Tested By:

Checked By:

Requested By:





CLIENT: URS & City of Tampa Water Department

7650 W. Courtney Campbell Causeway Address:

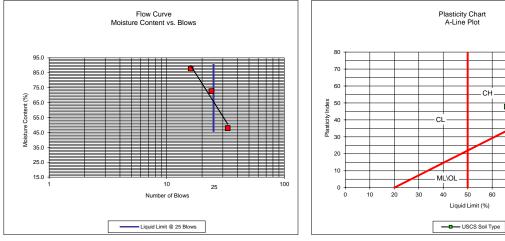
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II
Location:	Tampa, Florida

	Liquid Limit								
Number of	Number of Weight of Cont. Weight of Cont. Weight of Moisture								
Blows	+ Wet Soil	+ Dry Soil	Container	Content					
	(grams)	(grams)	(grams)	(%)					
33	21.26	20.71	19.56	47.8					
24	22.85	21.49	19.62	72.7					
16	22.79	21.30	19.60	87.6					

Plastic Limit							
Weight of Cont. Weight of Cont. Weight of Moisture							
+ Wet Soil	+ Dry Soil	Container	Content				
(grams)	(grams)	(grams)	(%)				
26.41	25.39	19.61	17.6				
25.95	24.98	19.79	18.7				



	Natural Moisture Content, Percent Passing #200 Sieve							
Cont+wet soil	Cont. + dry soil	Container wt.	Moisture Content	Cont+drv wash	% passing #200			
SOII	Cont. + dry son	Container wt.	Moisture Content	Cont+ury wash	% passing #200			
213.36	126.92	8.35	72.9%	29.37	82.3%			

NOTES:

Tested By: M. Hall Checked By: C. Suarez Boring #: SPT-6 Sample #: 12 Depth: 43.5-45

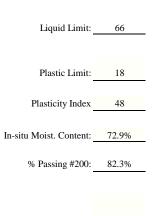
Date: March 12, 2019

Project #:

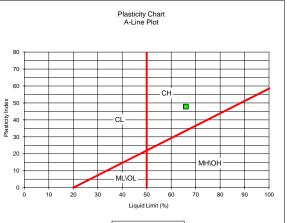
Requested By:

300881X3

C. Suarez



Results Summary





CLIENT: URS & City of Tampa Water Department

7650 W. Courtney Campbell Causeway Address:

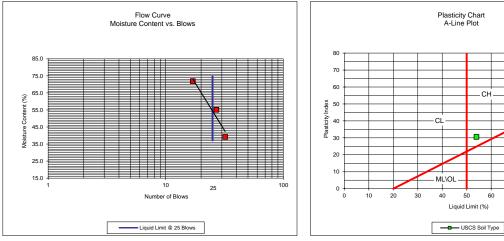
Suite 700

Tampa, Florida 33602-1462

Project:	Hillsborough River Dam Phase II
Location:	Tampa, Florida

Liquid Limit									
Number of	Weight of Cont.	Weight of Cont.	Weight of	Moisture					
Blows	+ Wet Soil	+ Dry Soil	Container	Content					
	(grams)	(grams)	(grams)	(%)					
32	21.26	20.81	19.66	39.1					
27	22.85	21.72	19.67	55.1					
17	22.79	21.51	19.73	71.9					

Plastic Limit								
Weight of Cont.	Weight of Cont.	Weight of	Moisture					
+ Wet Soil	+ Dry Soil	Container	Content					
(grams)	(grams)	(grams)	(%)					
27.12	25.76	19.72	22.5					
26.22	24.95	19.75	24.4					



Natural Moisture Content, Percent Passing #200 Sieve									
Cont+wet									
soil	Cont. + dry soil	Container wt.	Moisture Content	Cont+dry wash	% passing #200				
215.15	145.3	8.35	51.0%	64.21	59.2%				

NOTES:

Tested By: M. Hall Checked By: C. Suarez Boring #: SPT-6 Sample #: 22 Depth: 93.5-95

Date: March 12, 2019

Project #:

Requested By:

300881X3

C. Suarez

